

Appendix 5

Airport Pavement Assessments



DEPARTMENT OF
AVIATION

Pavement
Evaluation/
Maintenance
Management Program
2018



Oregon Department of Aviation

2018 Pavement Evaluation / Maintenance Management Program

Final Report – Individual Airports
Functional Category 2, Central Climatic Zone

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State of Oregon
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Introduction

The Oregon Department of Aviation has been collecting pavement condition information at eligible airports since the mid-1980s. In January 1995 the Federal Aviation Administration (FAA) mandated that any airport sponsor receiving and/or requesting federal funds for pavement improvement projects must have implemented a pavement maintenance management program. Through the Department's system planning efforts, the airports included in the Department's Pavement Evaluation / Maintenance Management Program have been complying with the intent of the law since the mid-1980s, well ahead of the FAA mandate. The information collected during this study ensures that your airport continues to comply with the Federal mandate. The developed pavement maintenance management program, as it relates to an individual airport, is described in this report.

The Oregon Department of Aviation routinely provides information to airport owners and operators throughout the State that assists them in maintaining and operating their airports. The State addresses many issues as part of their planning process, one of which is to provide to each individual airport, on a three-year cycle, a report on pavement condition. Through the statewide study, pavement maintenance management programs for all eligible airports in the state are efficiently and economically completed through the Department of Aviation's Pavement Evaluation / Maintenance Management Program.

Each airport owner or operator makes frequent decisions about the timing and type of maintenance and repair activities that should be completed on their pavements to maintain acceptable surface condition and adequate load-carrying capacity. The pavement maintenance management program described in this document, and supplemented by the information contained in the attached report prepared specifically for your airport, will assist you in making necessary decisions about pavement maintenance and rehabilitation projects at your airport, and will ensure compliance with the Federal mandate.

To develop a pavement maintenance management program for each eligible airport, the Department of Aviation elected to conduct pavement evaluations (visual inspections), and to implement the PAVER pavement maintenance management software. These activities were completed as part of the Department's Continuous Aviation System Plan efforts. PAVER uses the evaluation results to efficiently identify pavements requiring maintenance and rehabilitation, and to establish project priorities. The software can also be used to assess overall pavement network condition, prepare and forecast the budgets required to maintain the network at an acceptable condition level, and identify required maintenance and rehabilitation activities.

The federally mandated pavement maintenance management program identifies five major requirements:

- Pavement inventory
- Inspection schedule (detailed and monthly)
- Record keeping
- Information retrieval
- Program funding

The approach taken to meet these program requirements for your airport is described in this report.

Pavement Inventory

The FAA-mandated Pavement Inventory requirement specifies that information about each piece of pavement at an airport be compiled. This information is to include, at a minimum: pavement location, pavement dimensions, pavement surface type, and last construction date. The process used to develop this information is discussed under "Records Review".

Additionally, information is collected about the pavements at an airport so its pavement network can be defined. After the pavement network is defined, pavement inspections can be completed and a pavement maintenance management program can be developed. The methodology for defining the pavement network follows the Records Review discussion.

Records Review

The first step in meeting FAA's pavement maintenance management program requirement is to develop a maintenance and construction history for all pavements at an airport. For more than 30 years the Oregon Department of Aviation has, for its eligible airports, been conducting pavement evaluations to determine existing condition. In 1991 Pavement Consultants Inc. began assisting the Department in their efforts to compile and update that information. The information collected was used to develop a pavement maintenance management program for each eligible airport as described in this report, and your attached individual airport report.

Previous State-sponsored projects identified pavement layout, pavement construction history and pavement condition at each eligible airport. During this inspection cycle these documents were reviewed, and follow-up inquiries on pavement construction history were directed to the Oregon Department of Aviation, the FAA, consultants and airport sponsors. Based on this review, pavement boundaries were identified at your airport and were placed on an AutoCAD-generated base map (see Figure 1 in your attached airport report). The established base map fulfills the FAA "Pavement Inventory" requirement for locating pavements, identifying their dimensions, and identifying pavement type and age.

Network Definition

Once the pavement history at an airport has been compiled, individual pavement features can be identified, a process called network definition. These pavement features are defined on the basis of: primary use, construction history, and traffic pattern. Each airport is divided into features according to the guidelines contained in the current edition of ASTM International-Standard D5340, Standard Test Method for Airport Condition Index Surveys. The pavement features used in this project are defined as follows.

Network: Each eligible airport constitutes a separate pavement network.

<u>Branch</u>: A branch is any identifiable part of a pavement network that has a distinct function. Airfield pavements such as individual runways, taxiways and aprons are each considered to be a separate branch.

<u>Section</u>: A section is a subdivision of a branch and has consistent characteristics throughout its length or area. These characteristics include: pavement layer material type and thickness, construction history, traffic, and pavement condition. A section is the basic management unit of a pavement network, and is that portion of a branch over which a maintenance and rehabilitation project is likely to be completed.

<u>Sample Unit</u>: A sample unit is an arbitrarily defined portion of a pavement section that is used when performing detailed pavement inspections. It is the smallest subdivision in a pavement network. For flexible airport pavements such as asphalt concrete or surface treatment, sample units are about 5,000 square feet in area. For rigid (portland cement concrete) airport pavements, sample units typically include approximately 20 contiguous pavement slabs.

Beginning approximately 30 years ago, branches, sections and sample units were established for each eligible airport in the Oregon system. During this project, these divisions were reviewed and modified as required, based on changed conditions (new pavements, demolished pavements), or completion of any pavement-related maintenance and rehabilitation projects.

Branch and Section Names

Each pavement feature is assigned a name that allows it to be uniquely identified in the statewide airport system. Each branch name consists of a series of characters. The first character indicates the branch type: "R" for Runway, "T" for Taxiway, "A" for Apron or Helipad. The last two characters in the branch name identify the airport to which the branch belongs and were taken from the airport name. All branches for your airport carry this airport-specific two-letter identifier. The individual runway, taxiway, apron or helipad referenced is identified by characters located between the branch type ("R", "T" or "A") and your two-letter airport identifier. To the extent possible, these identifying characters were chosen to reflect the facility names you use. If the facility does not have a name it was assigned a number. In the case of runways, numbers are used that are the lower of the two runway numbers corresponding to compass bearing.

Located after a hyphen following the branch name are two- or three alpha-numeric characters. These characters identify the section within the branch. An example illustrating the naming convention is:

R17AU-01

which is the name for Runway 17/35, Aurora State Airport, Section 01.

The branches, sections and sample units identified for your airport are shown on Figure 2 in your attached individual airport report.

Network Identifiers

Several designators are used to describe information about a particular airport included in the State System Plan. These designators include: network identification, zone, functional category, funding group, ownership and climatic region.

Network Identification

Each airport in the statewide system is assigned a unique network identifier (name). This name is typically the name of the city in which the airport is located. The network identification name for your airport can be found in the appendices attached to your airport report. This network identification name is assigned so that an individual airport or a group of airports contained in the statewide database can be selected for evaluation. The statewide database contains information for all eligible airports in the State.

Zone

Zones are used to allow individual airports within the statewide database to be separately selected for analysis. The FAA airport designator is used as the zone designator.

Functional Category

Each airport is assigned a functional category based on its classification within the State System Plan. Each airport is assigned a functional category of either 1, 2, 3, 4 or 5 in accordance with the criteria set forth in the System Plan. These categories correspond to the following airport types: commercial service, business or high activity general aviation, regional general aviation, community general aviation, and low activity general aviation, respectively. The category assigned to your airport is listed in the appendices attached to your airport report. This category assignment allows groups of airports in different functional categories to be separately evaluated.

Funding Group

Airports in the State are categorized as either NPIAS or non-NPIAS. NPIAS designated airports are eligible for project funding under the FAA's Airport Improvement Program (AIP). Being designated as NPIAS or non-NPIAS in the database allows the Department to evaluate funding alternatives for the State airport system.

Ownership

Airport ownership is designated as Public, State or Private. This designation allows the Department to evaluate funding allocations based on eligibility for State and/or Federal funding.

Climatic Region

Each airport in the statewide system is assigned to one of three climatic regions - eastern, central or coastal. Because climatic conditions can impact pavement performance, assigning airports to a climatic region allows pavement performance to be more accurately modeled, resulting in more accurate pavement condition forecasts.

Branch or Section Identifiers

Several designators are used to describe a branch or section's function, importance or construction. These characteristics are: branch use, pavement rank, and surface type.

Branch Use

Branch use identifies the primary use of each distinct pavement area. For each airport pavement included in this study, a branch use of "Runway", "Taxiway", "Apron" or "Helipad" is assigned, as appropriate.

Pavement Rank

Pavement rank refers to the relative importance assigned to multiple facilities having the same branch use. Each pavement section is assigned a rank of primary ("P"), secondary ("S") or tertiary ("T") as appropriate. As an example, an airport with two runways might rank the more heavily used runway as primary and the lesser-used runway as secondary. The pavement rank assigned to each pavement section at your airport can be found in the appendices attached to your individual airport report.

Surface Type

Each pavement section is assigned a surface type designator based on the type of surface material present. Throughout the State six (6) surface types were encountered: asphalt overlay over asphalt concrete (AAC), asphalt concrete (AC), asphalt overlay over portland cement concrete (APC), portland cement concrete (PCC), surface treatment (ST), and chip seal (X). The surface type assigned to each pavement section at your airport is provided in the report appended to this document. Surface type identification fulfills one of FAA's "Pavement Inventory" requirements.

Structural and Construction History Data

Available construction records for each airport were obtained from the Oregon Department of Aviation, Federal Aviation Administration, consultants, or airport sponsors. These records were reviewed to establish a last construction date for each pavement section. Additional information was requested from individual airport sponsors to update or clarify this information, as necessary. The last construction date and known construction history for each pavement section can be found on Figure 1 in your individual airport report. The last construction date is also identified in the reports found in the attached appendixes. For those pavement sections where information

was not available, a last construction date was assigned based on pavement condition. Last construction date identification fulfills the final FAA "Pavement Inventory" requirement.

Field Verification

Information obtained through the records review and discussions with airport sponsors, Department of Aviation staff, FAA personnel and consultant staff was field-verified to ensure that each facility is accurately mapped and properly subdivided into branches and sections. Modifications to the maps, and/or branch and section divisions, were made as necessary wherever discrepancies in airport geometry, paving materials, or construction history were found during the visual inspections.

Inspection Schedule

The FAA's Pavement Maintenance Management Program guidelines require all airports seeking or receiving federal funds for pavement-related projects to complete both detailed and drive-by inspections. The guidelines require that detailed inspections be performed yearly, unless the inspections are conducted in accordance with the Pavement Condition Index methodology set forth in ASTM D5340, at which point detailed inspections are required once every three years. The Pavement Condition Index methodology is used to inspect Oregon's airports. Each airport is inspected on a three-year cycle thus complying with the FAA detailed inspection requirement.

The drive-by inspections required by the FAA are to be completed monthly. These inspections are cursory inspections that are performed to detect any unexpected changes in pavement condition.

A description of the detailed inspection methodology, as well as an approach to completing the monthly drive-by inspections, is provided below.

Detailed Inspection

Methodology

Pavement Condition Index (PCI) surveys were performed in May and July 2018 for all airports included in this year's project. The surveys were performed using the Pavement Condition Index (PCI) methodology developed by the U.S. Army Corps of Engineers, and outlined in the current edition of ASTM D-5340, *Standard Test Method for Airport Condition Index Surveys*. This document defines distress types, severity levels, and methods for measuring and recording distresses.

The PCI procedure was developed to collect data that would provide engineers and managers with a numerical value indicating overall pavement condition, and that would reflect both pavement structural integrity and surface operational condition. The procedure was designed to be highly repeatable and was found to be well-correlated with the judgment of experienced pavement engineers.

A PCI survey is performed by measuring the amount and severity of certain defined distresses (defects) observed in a sample unit. Table 1 lists both the asphalt concrete and portland cement concrete pavement distress types considered in the PCI method, and also identifies their most common cause (load, climate/durability, other) as assigned by the PAVER software. Load-related distresses are apparent where the pavement has been over-stressed by traffic loads applied to its surface. Climate/durability-related distresses arise due to exposure to the environment. Other-related distresses are caused by actions not related to load or climate such as fuel spills or construction deficiencies.

Table 1. Pavement Condition Index Distress

Types and Related Causes.

Asphalt Cor	icrete	Portland Cement Concrete		
Pavement Distress	Related Cause	Pavement Distress	Related Cause	
Alligator Cracking	Load	Blow-Up	Climate/Durability	
Bleeding	Other	Corner Break	Load	
Block Cracking	Climate/Durability	Cracks: Longitudinal, Transverse, and Diagonal	Load	
Corrugation	Other	Durability ("D") Crack	Climate/Durability	
Depression	Other	Joint Seal Damage	Climate/Durability	
Jet Blast Erosion	Other	Patching, Small	Other	
Joint Reflection Cracking	tion Cracking Climate/Durability Patching Util		Other	
Longitudinal and Transverse Cracking	Climate/Durability	Popouts	Other	
Oil Spillage	Other	Pumping	Other	
Patching and Utility Cut Patching	Climate/Durability	Scaling, Map Cracking, Crazing	Other	
Polished Aggregate	Other	Settlement or Faulting	Other	
Raveling	Climate/Durability	Shattered Slab / Intersecting Cracks	Load	
Rutting	Load	Shrinkage Cracks	Other	
Shoving	Other	Spalling (Longitudinal and Transverse Joint)	Other	
Slippage Cracking	Other	Spalling (Corner)	Other	
Swell Other		Alkali Silica Reaction (ASR)	Other	
Weathering	Climate/Durability			

To obtain a statistically reliable PCI for a given pavement section it is not necessary to inspect all sample units in that section. A pre-determined number of randomly chosen sample units are selected for inspection based on the total number of sample units in the section. The sampling rates used during this study are shown in Table 2. The sampling rates contained in Table 2 result in data that are reliable at a 92 percent confidence level.

Table 2. Selection of Number of Sample Units to Inspect.

Flexible P	avement	Rigid Pa	vement
N	n	N	n
1	1	1	1
2 - 3	2	2	2
4 - 6	3	3 - 4	3
7 - 13	4	5 - 6	4
14 - 38	5	7 - 8	5
39 +	6	9 - 11	6
		12 - 14	7
		15 - 19	8
		20 - 27	9
		28 - 38	10
		39 - 58	11
		59 - 104	12
		105 - 313	13
		314 +	14

Where: N = Total number of sample units in a pavement section

n = Number of sample units to be surveyed

Pavement Condition Index Calculation

To calculate a PCI for a given sample unit, each distress type observed is assigned a deduct value based on its density (frequency of occurrence) in that sample area, and its severity. All deducts are summed and subsequently adjusted (corrected) for the number of different distresses found. This corrected deduct value is subtracted from 100, the PCI for a "perfect" pavement, to arrive at a PCI for that particular sample unit. The PCI for a pavement section is the area-weighted average PCI value of all sample units evaluated in that section. Pavement Condition Ratings (PCRs) are associated with ranges of PCI values.

The color-coded Figure 3 in your attached individual airport report shows the PCRs and their associated PCI ranges, as well as the pavement condition at your airport in May or July 2018.

Monthly Drive-By Inspection

As part of the FAA-mandated pavement maintenance management program, a monthly drive-by inspection is required. This inspection is intended to identify abrupt changes in condition occurring since the last monthly inspection, and to record any maintenance activities completed during the previous month. This inspection can easily be accomplished by driving your airport and noting any changes or maintenance performed on the form provided in Figure 1. Each drive-by inspection must note the date the inspection was completed, and record any maintenance performed since the last inspection. These records must be kept on-file for five years.

Figure 1. Monthly Drive-By Inspection Form.

Airport:	 	 	
Date:	 	 	
nspector: _			

Branch*	Section*	Distresses Observed	Maintenance Performed Since Last Inspection

^{*} Refer to the "Airport Layout, Dimensions and Pavement Cross-Sections" or "Pavement Branch, Section and Sample Unit Layout" figures in your airport report.

Record Keeping and Data Retrieval

The FAA pavement maintenance management program requires that compiled records be kept for five years. To facilitate record keeping and data retrieval at the State level, the PAVER pavement maintenance management software was implemented. PAVER provides the Oregon Department of Aviation with a method for storing data and generating reports.

PAVER was developed by the U.S. Army Construction Engineering Research Laboratory (USA-CERL). The program uses the guidelines contained in the current edition of ASTM D5340 as its basis. The current version, Version 7.0.6, is a Windows-based program that can store pavement condition information, as well as construction and maintenance history information. Using the data stored in the PAVER database the user has many capabilities, including: evaluating current condition, predicting future condition, determining maintenance and rehabilitation needs, scheduling future inspections, and preparing budget estimates.

The statewide database containing the information for all evaluated airports was updated during this project. Information for each individual airport can easily be extracted from the statewide database. The database allows required records to be stored indefinitely, thus meeting the FAA requirement that records be maintained for a five-year period. Additionally, the software allows data to be retrieved quickly and efficiently.

After data were entered into the State's PAVER database for each inspected airport, the software was used to analyze the stored data and to generate useful reports. The reports described in Table 3 were generated for your airport and are provided as appendices to your individual airport report.

Table 3. PAVER Reports.

Report Name	Report Description
Branch Condition	Lists information about each branch, including: network identification, branch identification, name, use, number of sections, total branch area and the average and area-weighted average PCI for the entire branch.
Section Condition	Provides information about each section, including: branch identification and section number, last construction date, surface type, use, rank, section area, last inspection date, age of pavement at last inspection and the PCI at the last inspection.
Network Maintenance	Applies the stored distress maintenance policy to the pavement network and identifies the type and cost of routine maintenance required across the entire network. Information in this report is listed by section.
Re-Inspection	Summarizes the distress data collected during the most recent inspection and provides the PCI for each sample unit inspected, as well as summary information about the section.

Pavement Condition Prediction

To allow future pavement condition to be predicted, data collected throughout the State were used to generate "performance curves". The curves were developed based on surface type, use, airport functional category and climatic region. These curves (models) are used to predict future pavement condition by assuming the behavior of an individual pavement section is similar to the behavior of the pavement sections used to generate the "performance curve". Figures 2 through 10 show the "performance curves" used to model pavements in your airport's functional category and climatic region.

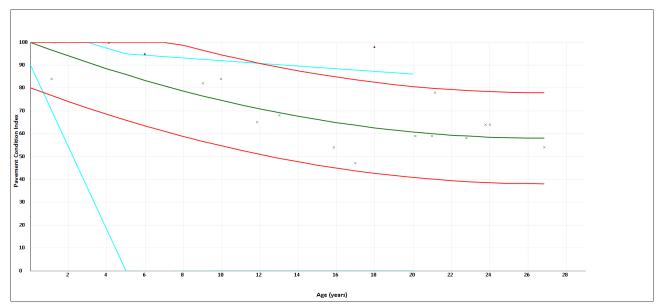


Figure 2. Performance Curve for Category 2 AAC Aprons – Central Oregon.

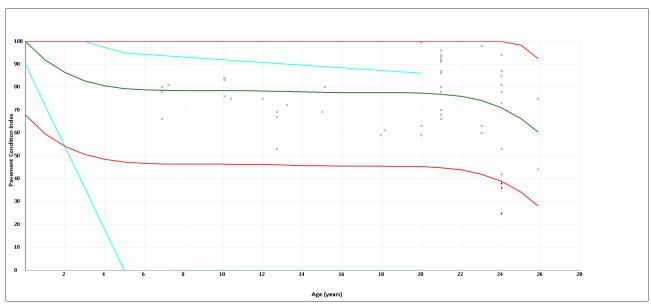


Figure 3. Performance Curve for Category 2 AAC Runways – Central Oregon.

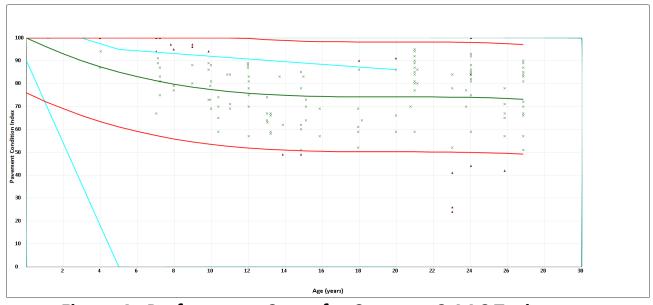


Figure 4. Performance Curve for Category 2 AAC Taxiways – Central Oregon.

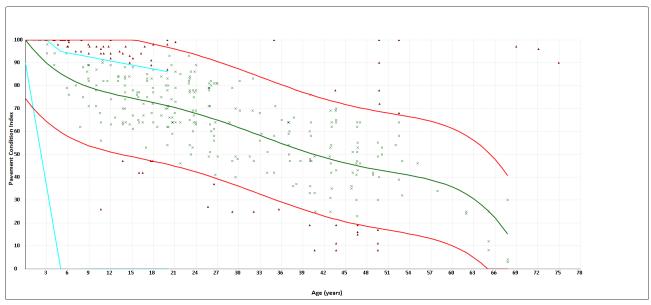


Figure 5. Performance Curve for Category 2 AC Aprons – Central Oregon.

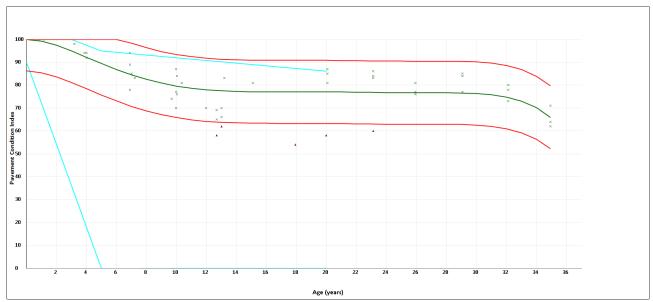


Figure 6. Performance Curve for Category 2 AC Runways – Central Oregon.

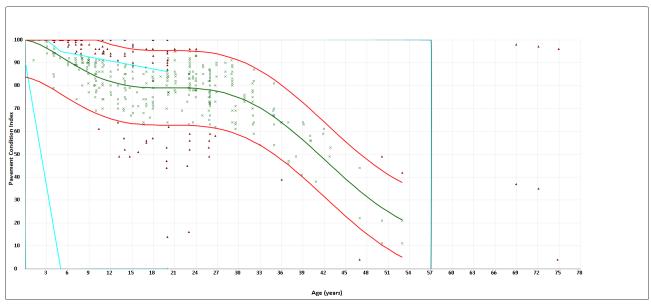


Figure 7. Performance Curve for Category 2 AC Taxiways – Central Oregon.

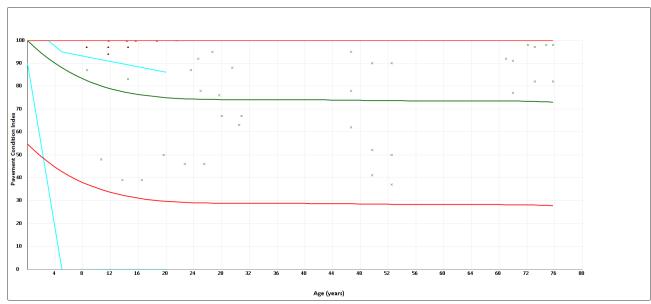


Figure 8. Performance Curve for Category 2 PCC Aprons – Central Oregon.

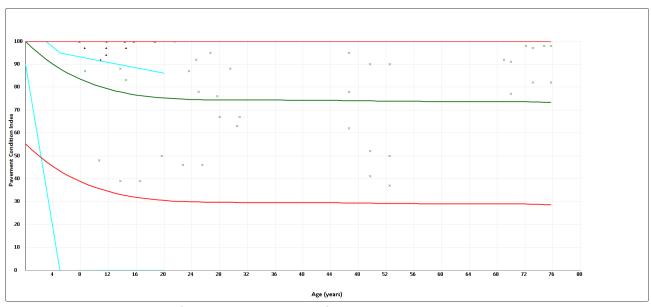


Figure 9. Performance Curve for Category 2 PCC Taxiways – Central Oregon.

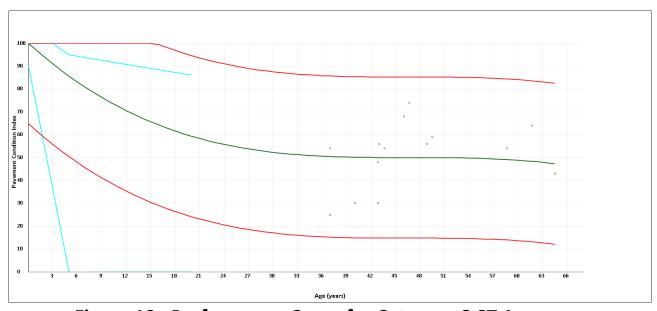


Figure 10. Performance Curve for Category 2 ST Aprons – Central Oregon.

Typical Maintenance Requirements

The PAVER-generated M&R Plan Report was used to identify when pavement maintenance and rehabilitation projects are required for a given pavement section, and what repair type is most appropriate. The repair strategies evaluated were:

- Reconstruction (pavements with Pavement Condition Indices less than 40).
- Overlay flexible pavements (runways with Pavement Condition Indices between 40 and 65, taxiways between 40 and 60, aprons between 40 and 55, and pavements exhibiting significant load-related distress with PCIs above the critical PCI).
- Global maintenance (fog seal, slurry seal or thin (2 inch) overlay) applied on a user-specified interval of 6 years for a fog seal, 6 years for a slurry seal, and 10 years for an overlay, unless the Pavement Condition Index (PCI) is above 90, at which point the global maintenance will be scheduled when the PCI falls to 90 or below. The global maintenance type recommended is based on the distress types observed in the section during the visual inspections.
- Routine maintenance, such as crack sealing and patching.

The M&R Plan Report was generated for a 5-year period beginning in June 2019. Included in the work plan are estimated costs for each recommended project. The costs are estimated by applying a unit cost for the recommended activity to the square foot area of the pavement section. The unit costs include adjustments for engineering and administration, mobilization, restriping and contingency. The unit costs used to develop the work plan activity cost are shown in Table 4. The recommended work plan for your airport is provided in your attached individual airport report.

Table 4. Unit Costs for the Various Work Plan Activities.

Activity	Unit	Unit Cost
Fog Seal	SF	\$0.19
Slurry Seal	SF	\$0.31
2" Asphalt Concrete Overlay	SF	\$2.50
2" – 3" AC Mill and Replace	SF	\$3.00 - \$4.50
Reconstruction	SF	\$7.95 – 13.6

AURORA STATE AIRPORT

This report describes how your Pavement Maintenance Management Program (PMMP) was developed. Your Program was developed as part of the Oregon Continuous Aviation System Plan sponsored in part by the Oregon Department of Aviation and the Federal Aviation Administration (FAA). The information and data contained in this report ensures you comply with the requirements of FAA Grant Assurance Number 11 which states that any airport requesting federal funds for pavement improvement projects must have implemented a pavement maintenance management program.

DATA COLLECTION

To determine how your pavements were constructed and their age, a records review was conducted. Figure AU-1 shows the records review results. This figure identifies pavement boundaries, dimensions, pavement layer types, thicknesses and dates of construction. The most recent construction date for each pavement can also be found in the Section Condition Report in Appendix 2. Figure AU-1 and the information contained in Appendices 1, 2 and 4 ensure that your airport complies with the "pavement inventory" requirement of FAA's PMMP guidelines.

The pavements at your airport were divided into branches, sections and sample units in accordance with the methodology outlined in the current edition of ASTM D5430, *Standard Test Method for Airport Condition Index Surveys*. The branches, sections and sample units established at your airport are shown in Figure AU-2. A Branch Condition Report showing all branches, their associated areas, and their area-weighted average condition is provided in Appendix 1. Additionally, the Appendix 2 Section Condition Report provides information used to define each branch and section in the PAVER database.

Using the branch, section and sample unit divisions established, a visual condition survey was conducted at Aurora State Airport in July 2018. During the inspection, pavement defects were identified and measured in accordance with the methodology outlined in ASTM D5430. This inspection ensures your airport complies with the "detailed inspection" requirement of FAA's PMMP guidelines. After collection, the data were entered into the PAVER software for analysis. These data are reproduced in the Re-Inspection Report attached as Appendix 4.

The PAVER database updated during this project ensures your airport complies with the "record keeping and information retrieval" requirements of FAA's PMMP guidelines.

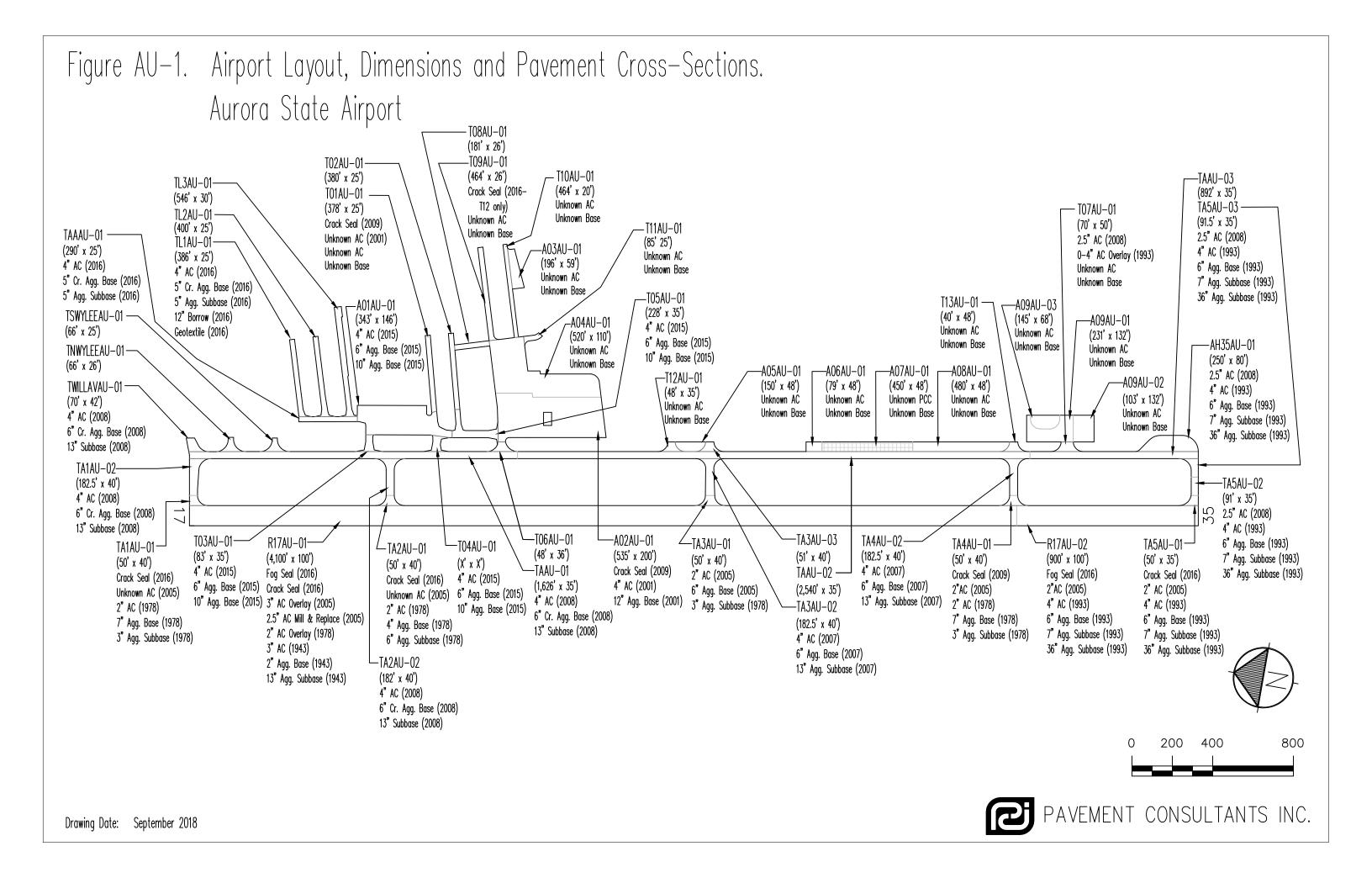
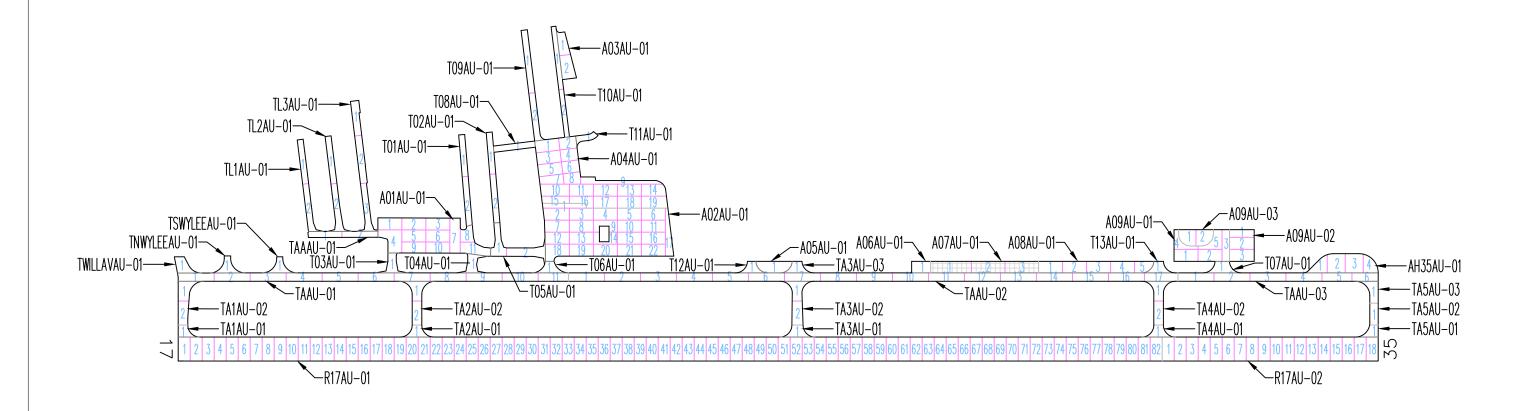
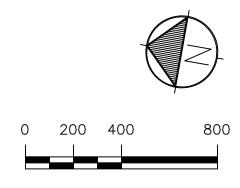


Figure AU-2. Pavement Branch, Section and Sample Unit Layout.

Aurora State Airport







RESULTS

Using the data collected during the visual inspection, the PAVER software was used to calculate an area-weighted average Pavement Condition Index (PCI) for each pavement section inspected using the sample units evaluated. Using each section's PCI, a Pavement Condition Rating (PCR) was assigned. The PCIs measured during this inspection are shown in Table 1. The table also contains PCIs from past inspections as well as projected PCIs for 2023 and 2028. The projections were based on pavement deterioration models developed by PAVER using the inspection data from other pavements in the same airport category as your airport, located in the same climatic region, and with the same surface type and use.

The Branch Condition Report in Appendix 1 summarizes current pavement condition by branch while the Section Condition Report in Appendix 2 lists pavement condition by section. The current Pavement Condition Rating (PCR) is shown graphically in Figure AU-3.

Table 1. Past, Present and Future Pavement Condition Indices.

Duomah	Castian		Inspections		Fore	ecast
Branch	Section	2012	2015	2018	2023	2028
A01AU	01		100	100	85	78
A02AU	01	82	64	53	48	44
A03AU	01	78	53	49	45	42
A04AU	01	98	62	68	63	57
A05AU	01	69	41	40	35	25
A06AU	01	100	86	82	77	73
A07AU	01	87	95	88	81	77
A08AU	01	78	64	70	66	60
A09AU	01	60	64	49	45	42
A09AU	02		100	75	72	68
A09AU	03		100	88	80	75
AH35AU	01	100	80	71	67	62
R17AU	01	83	81	83	78	77
R17AU	02	81	75	72	47	24
T01AU	01	95	89	88	81	79
T02AU	01	91	85	74	65	52
T03AU	01		100	100	93	84
T04AU	01		100	100	93	84
T05AU	01		100	100	93	84
T06AU	01	100	89	80	79	79
T07AU	01	100	91	79	75	74
T08AU	01	83	80	64	51	37
T09AU	01	86	73	71	60	46
T10AU	01	78	58	61	48	34

Table 1. Past, Present and Future Pavement Condition Indices.

Duomak	Section		Inspections		Fore	ecast
Branch	Branch Section		2015	2018	2023	2028
T11AU	01	64	62	69	58	43
T12AU	01	96	79	66	54	39
T13AU	01	84	80	63	50	36
TA1AU	01	100	70	59	58	56
TA1AU	02	94	89	88	81	79
TA2AU	01	81	74	67	66	64
TA2AU	02	100	92	89	82	79
TA3AU	01	75	65	66	65	63
TA3AU	02	100	92	80	79	79
TA3AU	03	100	90	88	81	79
TA4AU	01	83	59	58	57	55
TA4AU	02	92	80	74	65	52
TA5AU	01	74	81	49	35	24
TA5AU	02	100	90	69	58	43
TA5AU	03		89	73	72	70
TAAAU	01			100	93	84
TAAU	01	100	92	83	79	79
TAAU	02	100	91	73	63	50
TAAU	03	100	89	69	58	43
TL1AU	01			100	93	84
TL2AU	01			100	93	84
TL3AU	01			100	93	84
TNWYLEEAU	01	100	94	75	67	54
TSWYLEEAU	01	100	94	94	85	80
TWILLAVAU	01	100	94	89	82	79

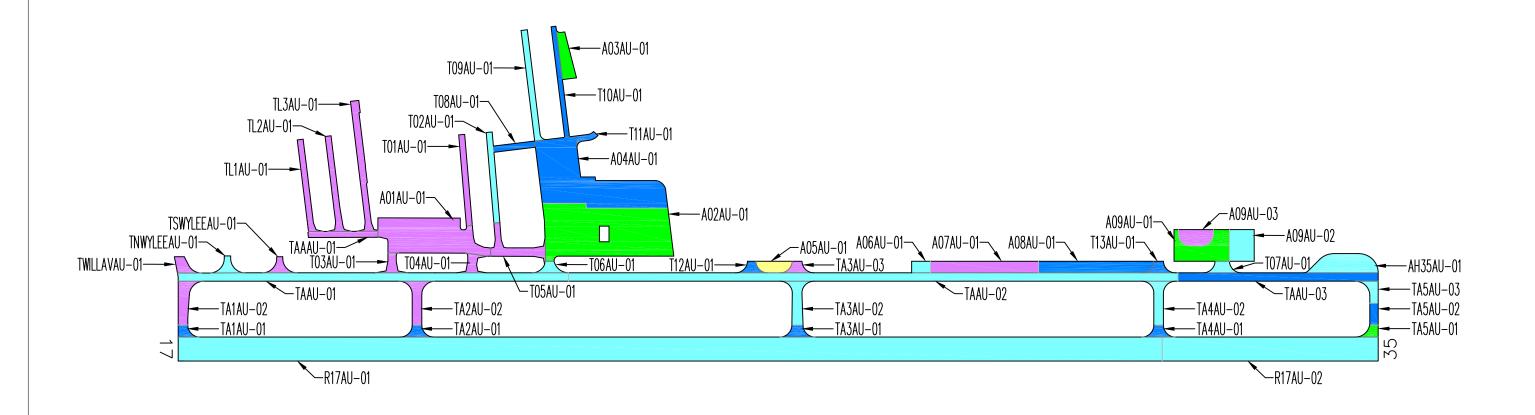
Section PCIs at Aurora State Airport range from a low of 40 (a PCR of "Very Poor") to a high of 100 (a PCR of "Good"). The area-weighted average PCI for all airport pavements is 77, corresponding to an overall PCR of "Satisfactory". Figure AU-4 shows how much pavement area is associated with each Pavement Condition Rating category and also shows pavement condition distribution from the inspections conducted in 2012 and 2015.

The primary distresses observed during the inspection were: longitudinal and transverse cracking, weathering, patching, block cracking, alligator cracking, raveling and depressions. The primary distress observed in the concrete pavement was joint spalls, with isolated occurrences of linear cracking.

A graphical representation of the projected PCIs listed in Table 1 is shown in Figure AU-5.

Figure AU-3. Pavement Condition in July 2018.

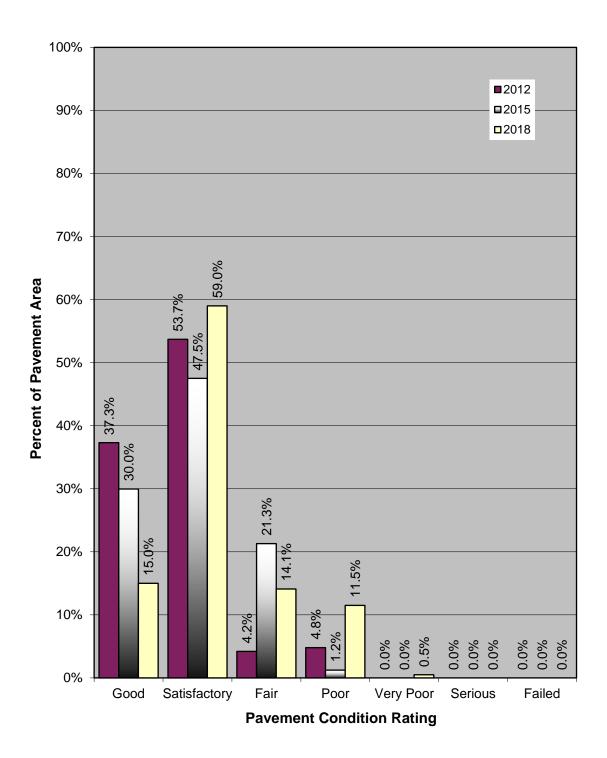
Aurora State Airport





Drawing Date: September 2018

Figure AU-4. Pavement Condition Distribution Aurora State Airport



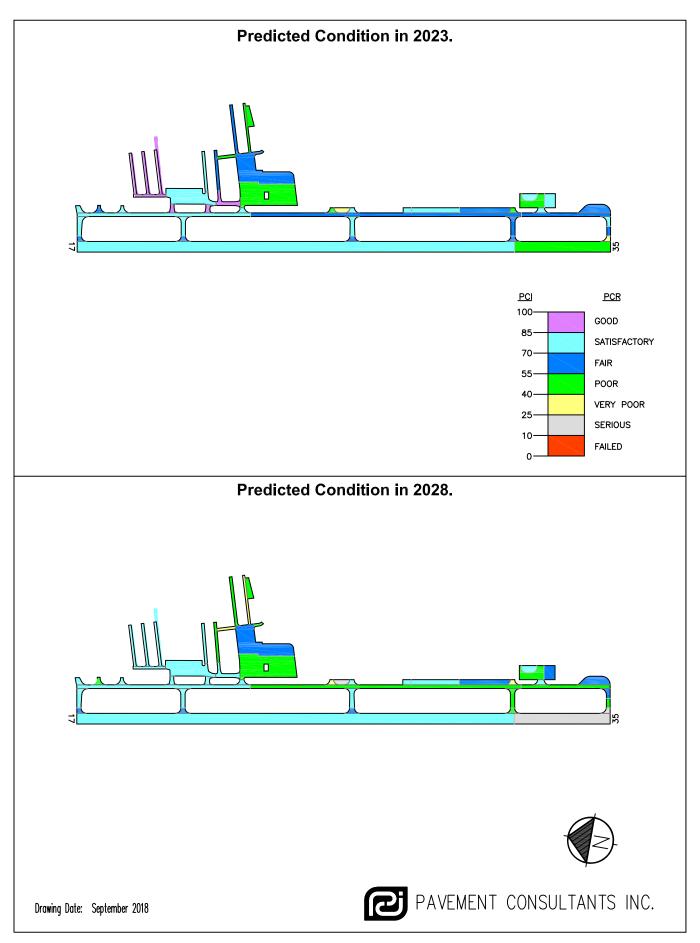


Figure AU-5. Future Pavement Condition.

RECOMMENDATIONS

Data collected during the visual condition survey were used by the PAVER software to generate the Network Maintenance Report contained in Appendix 3. This report identifies, for each pavement section, the recommended localized maintenance activities (i.e.-crack sealing, patching) that should be completed to repair the defects observed during the visual inspection. The repair quantities identified in the report were extrapolated to cover the entire pavement section, based on the distresses measured in the inspected sample units. If the repair activities identified are completed, the pavement deterioration rate will be slowed.

The recommended localized maintenance activities to be applied are selected by the PAVER software based on a Distress Maintenance Policy established for the Oregon airport system. The report results indicate that, over your entire airport, the following quantities of localized maintenance are needed:

- 20,295 linear feet of asphalt concrete crack sealing
- 4 linear feet of asphalt concrete wide crack sealing/repair.

The PAVER software can also identify and schedule recommended global (applied over an entire section) maintenance activities such as fog seals, slurry seals and other surface treatments, as well as major rehabilitation activities such as asphalt concrete overlays and complete reconstruction. PAVER schedules global maintenance on a user-defined interval. To schedule major rehabilitation PAVER uses pavement deterioration models developed during this project. These models are used to estimate future pavement condition and to schedule rehabilitation based on a trigger PCI.

During this project a 5-year program outlining recommended global maintenance and rehabilitation was developed. The program begins in the year 2019 to allow time for project development. These recommendations are presented in Table 2, which identifies the pavement section requiring rehabilitation, the year the action should be completed, the type of action, and an associated cost. This information is also presented graphically in Figure AU-6.

Table 2. Five-Year Global Maintenance and Rehabilitation Plan.

Year	Branch	Section	Action	Area (sf)	Unit Cost (\$/sf)	Total Cost (\$)
2019	A02AU	01	Slurry Seal	109,649	\$0.31	\$33,991
2019	A03AU	01	2" AC Overlay	9,162	\$2.50	\$22,905
2019	A04AU	01	Slurry Seal	87,212	\$0.31	\$27,036
			4" AC over 6" Crushed			
2019	A05AU	01	Aggregate Base over	6,184	\$11.45	\$70,807
			13" Aggregate Subbase			

Table 2. Five-Year Global Maintenance and Rehabilitation Plan.

				Area	Unit Cost	Total Cost
Year	Branch	Section	Action	(sf)	(\$/sf)	(\$)
2019	A06AU	01	Slurry Seal	3,790	\$0.31	\$1,175
2019	A08AU	01	Slurry Seal	22,503	\$0.31	\$6,976
2019	A09AU	01	2" AC Overlay	21,705	\$2.50	\$54,263
2019	A09AU	02	Slurry Seal	13,596	\$0.31	\$4,215
2019	A09AU	03	Slurry Seal	8,786	\$0.31	\$2,724
2019	AH35AU	01	Slurry Seal	19,308	\$0.31	\$5,985
2019	T01AU	01	Slurry Seal	9,478	\$0.31	\$2,938
2019	T02AU	01	Slurry Seal	9,468	\$0.31	\$2,935
2019	T06AU	01	Slurry Seal	3,128	\$0.31	\$970
2019	T07AU	01	Slurry Seal	3,953	\$0.31	\$1,225
2019	T08AU	01	Slurry Seal	4,516	\$0.31	\$1,400
2019	T09AU	01	Slurry Seal	12,198	\$0.31	\$3,781
2019	T10AU	01	2" AC Overlay	9,280	\$2.50	\$23,200
2019	T11AU	01	Slurry Seal	2,325	\$0.31	\$721
2019	T12AU	01	Slurry Seal	2,749	\$0.31	\$852
2019	T13AU	01	Slurry Seal	2,992	\$0.31	\$928
2019	TA1AU	01	2" AC Overlay	2,537	\$2.50	\$6,343
2019	TA1AU	02	Slurry Seal	8,740	\$0.31	\$2,709
2019	TA2AU	01	Slurry Seal	3,073	\$0.31	\$953
2019	TA2AU	02	Slurry Seal	8,595	\$0.31	\$2,664
2019	TA3AU	01	Slurry Seal	3,403	\$0.31	\$1,055
2019	TA3AU	02	Slurry Seal	8,813	\$0.31	\$2,732
2019	TA3AU	03	Slurry Seal	3,190	\$0.31	\$989
2019	TA4AU	01	2" AC Overlay	3,324	\$2.50	\$8,310
2019	TA4AU	02	Slurry Seal	9,028	\$0.31	\$2,799
2019	TA5AU	01	2" AC Overlay	2,520	\$2.50	\$6,300
2019	TA5AU	02	Slurry Seal	3,188	\$0.31	\$988
2019	TA5AU	03	Slurry Seal	3,975	\$0.31	\$1,232
2019	TAAU	01	Slurry Seal	56,785	\$0.31	\$17,603
2019	TAAU	02	Slurry Seal	88,885	\$0.31	\$27,554
2019	TAAU	03	Slurry Seal	29,204	\$0.31	\$9,053
2019	TNWYLEEAU	01	Slurry Seal	3,465	\$0.31	\$1,074
2019	TWILLAVAU	01	Slurry Seal	3,777	\$0.31	\$1,171
					2019 Total	\$362,556
2021	R17AU	02	Slurry Seal	90,000	\$0.31	\$27,900
2021	TSWYLEEAU	01	Fog Seal	3,237	\$0.19	\$615
2021	A01AU	01	Fog Seal	56,334	\$0.19	\$10,703
2021	R17AU	01	Slurry Seal	410,000	\$0.31	\$127,100

Table 2. Five-Year Global Maintenance and Rehabilitation Plan.

Year	Branch	Section	Action	Area (sf)	Unit Cost (\$/sf)	Total Cost (\$)
2021 Total						\$166,318
5-Year Total						\$528,875

If the global maintenance and/or rehabilitation activities recommended in Table 2 are not completed, the localized maintenance activities identified in the Network Maintenance Report (Appendix 3) for that section should be done. Additionally, for those sections not listed in Table 2 as requiring global maintenance or rehabilitation, the localized maintenance activities outlined in the Network Maintenance Report should be completed. By completing the localized maintenance activities, pavement condition is improved, life is extended, deterioration is slowed and the length of time until major repair or rehabilitation is required is increased.

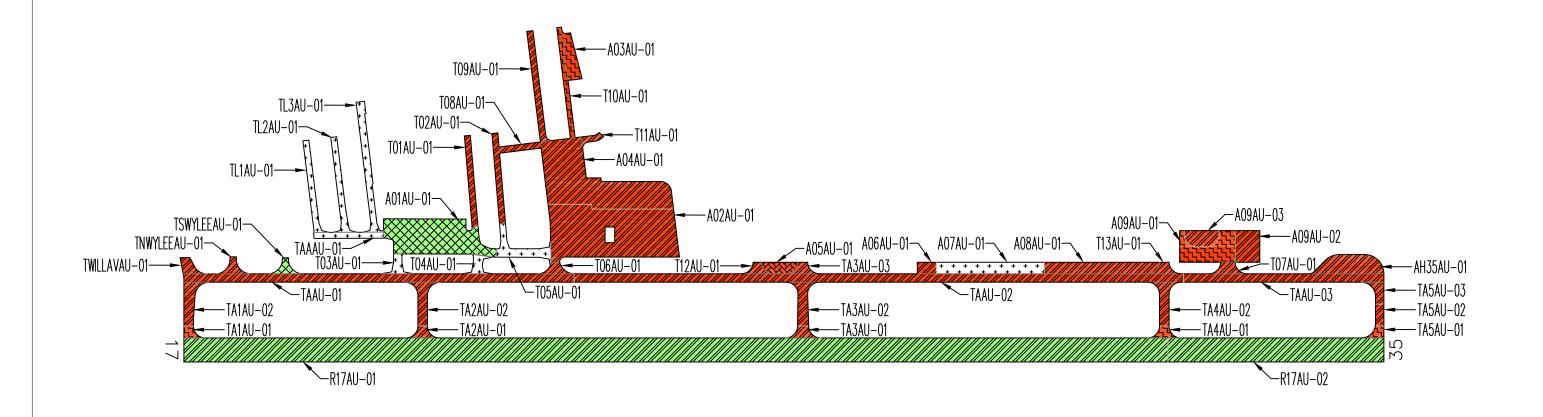
INSPECTION SCHEDULE

To comply with the inspection schedule requirement of FAA Grant Assurance Number 11, a detailed visual inspection should be conducted every 3 years using the methodology described in ASTM D5430. The next scheduled detailed visual inspection should take place in 2021.

In addition, the FAA requires that a drive-by inspection be conducted monthly to detect unforeseen changes in pavement condition. The results of each drive-by inspection should be recorded and kept in a file. At a minimum, the date of the inspection and an indication of any maintenance performed since the last drive-by inspection should be recorded.

Figure AU-6. Five-Year Pavement Management Plan.

Aurora State Airport







Appendix 1 Branch Condition Report

Branch Condition Report

Page 1 of 2

Pavement Database: ODA_2018 _Final

Branch ID	Number of Sections	Sum Section Length (Ft)	Avg Section Width (Ft)	True Area (SqFt)	Use	Average PCI	Standard Deviation PCI	Weighted Average PCI
A01AU	1	343.00	146.00	56,334.00	APRON	100.00	0.00	100.00
A02AU	1	523.00	200.00	109,649.00	APRON	53.00	0.00	53.00
A03AU	1	197.00	59.00	9,162.00	APRON	49.00	0.00	49.00
A04AU	1	520.00	280.00	87,212.00	APRON	68.00	0.00	68.00
A05AU	1	150.00	48.00	6,184.00	APRON	40.00	0.00	40.00
A06AU	1	79.00	48.00	3,790.00	APRON	82.00	0.00	82.00
A07AU	1	450.00	48.00	21,600.00	APRON	88.00	0.00	88.00
A08AU	1	480.00	48.00	22,503.00	APRON	70.00	0.00	70.00
A09AU	3	479.00	110.67	44,087.00	APRON	70.67	16.21	64.79
AH35AU	1	225.00	80.00	19,308.00	APRON	71.00	0.00	71.00
R17AU	2	5,000.00	100.00	500,000.00	RUNWAY	77.50	5.50	81.02
T01AU	1	380.00	25.00	9,478.00	TAXIWAY	88.00	0.00	88.00
T02AU	1	378.00	25.00	9,468.00	TAXIWAY	74.00	0.00	74.00
T03AU	1	83.00	35.00	3,684.00	TAXIWAY	100.00	0.00	100.00
T04AU	1	75.00	40.00	3,880.00	TAXIWAY	100.00	0.00	100.00
T05AU	1	228.00	35.00	11,678.00	TAXIWAY	100.00	0.00	100.00
T06AU	1	48.00	36.00	3,128.00	TAXIWAY	80.00	0.00	80.00
T07AU	1	48.00	60.00	3,953.00	TAXIWAY	79.00	0.00	79.00
T08AU	1	174.00	25.00	4,516.00	TAXIWAY	64.00	0.00	64.00
T09AU	1	464.00	26.00	12,198.00	TAXIWAY	71.00	0.00	71.00
T10AU	1	464.00	20.00	9,280.00	TAXIWAY	61.00	0.00	61.00
T11AU	1	85.00	25.00	2,325.00	TAXIWAY	69.00	0.00	69.00
T12AU	1	48.00	35.00	2,749.00	TAXIWAY	66.00	0.00	66.00
T13AU	1	40.00	48.00	2,992.00	TAXIWAY	63.00	0.00	63.00
TA1AU	2	232.50	40.00	11,277.00	TAXIWAY	73.50	14.50	81.48
TA2AU	2	232.50	40.00	11,668.00	TAXIWAY	78.00	11.00	83.21
TA3AU	3	283.50	40.00	15,406.00	TAXIWAY	78.00	9.09	78.56
TA4AU	2	232.50	40.00	12,352.00	TAXIWAY	66.00	8.00	69.69
TA5AU	3	232.50	35.00	9,683.00	TAXIWAY	63.67	10.50	65.44
TAAAU	1	290.00	25.00	7,284.00	TAXIWAY	100.00	0.00	100.00
TAAU	3	5,000.00	35.00	174,874.00	TAXIWAY	75.00	5.89	75.58
TL1AU	1	386.00	25.00	9,921.00	TAXIWAY	100.00	0.00	100.00
TL2AU	1	400.00	25.00	10,673.00	TAXIWAY	100.00	0.00	100.00
TL3AU	1	546.00	25.00	15,963.00	TAXIWAY	100.00	0.00	100.00
TNWYLEE	1	66.00	26.00	3,465.00	TAXIWAY	75.00	0.00	75.00
TSWYLEE	1	66.00	25.00	3,237.00	TAXIWAY	94.00	0.00	94.00
TWILLAVA	1	70.00	42.00	3,777.00	TAXIWAY	89.00	0.00	89.00

7/26/2018	Branch Condition Report	Page 2 of 2
	Pavement Database: ODA_2018 _Final	

Use Category	Number of Sections	Total Area (SqFt)	Arithmetic Average PCI	Average STD PCI	Weighted Average PCI
APRON	12	379829.000466786	69.42	17.77	68.68
RUNWAY	2	500000.002380733	77.50	5.50	81.02
TAXIWAY	35	368909.0000595	78.80	14.44	79.61
ALL	49	1248738.00290702	76.45	15.61	76.85

Appendix 2 Section Condition Report

Pavement Database: ODA_2018 _Final

NetworkId: Aurora

Pavement Date	abase: ODA_201	is _rinal			Netu	orkid.	Aurora			
Branch ID	Section ID	Last Const. Date	Surface	Use	Rank	Lanes	True Area (SqFt)	Last Inspection Date	Age At Inspec tion	
A01AU	01	9/26/2015	AC	APRON	Р	0	56,334.00	7/12/2018	3	100
A02AU	01	8/2/2001	AC	APRON	Р	0	109,649.00	7/12/2018	17	53
A03AU	01	1/1/1969	AC	APRON	S	0	9,162.00	7/12/2018	49	49
A04AU	01	1/1/2008	AC	APRON	Р	0	87,212.00	7/12/2018	10	68
A05AU	01	1/1/1989	AC	APRON	S	0	6,184.00	7/12/2018	29	40
A06AU	01	1/1/2007	AC	APRON	S	0	3,790.00	7/12/2018	11	82
A07AU	01	1/1/1989	PCC	APRON	S	0	21,600.00	7/12/2018	29	88
A08AU	01	1/1/1989	AC	APRON	S	0	22,503.00	7/12/2018	29	70
A09AU	01	1/1/1989	AC	APRON	S	0	21,705.00	7/12/2018	29	49
A09AU	02	6/1/2010	AC	APRON	S	0	13,596.00	7/12/2018	8	
A09AU	03	6/1/2010	AC	APRON	S	0	8,786.00	7/12/2018	8	88
AH35AU	01	8/1/2008	AC	APRON	Р	0	19,308.00	7/12/2018	10	71
R17AU	01	5/2/2005	AC	RUNWAY	Р	0	410,000.00	7/12/2018	13	
R17AU	02	5/1/2005	<u> </u>	RUNWAY	P	0	90,000.00	7/12/2018	13	
T01AU	01	8/1/2001	AC	TAXIWAY	S	0	9,478.00	7/12/2018		
T02AU	01	8/1/2001	AC	TAXIWAY	S	0	9,468.00	7/12/2018	-	<u>. </u>
T03AU	01	9/26/2015	AC	TAXIWAY	S	0	3,684.00	7/12/2018	3	100
T04AU	01	9/26/2015	AC	TAXIWAY	S	0	3,880.00	7/12/2018	3	100
T05AU	01	9/26/2015	AC	TAXIWAY	S	0	11,678.00	7/12/2018	3	100
T06AU	01	9/3/2008	AC	TAXIWAY	S	0	3,128.00	7/12/2018	10	80
T07AU	01	8/1/2008	AAC	TAXIWAY	S	0	3,953.00	7/12/2018	10	79
T08AU	01	1/1/1989	AC	TAXIWAY	S	0	4,516.00	7/12/2018	29	64
T09AU	01	1/1/1989	AC	TAXIWAY	S	0	12,198.00	7/12/2018	29	71
T10AU	01	1/1/1989	AC	TAXIWAY	S	0	9,280.00	7/12/2018	29	61
T11AU	01	1/1/1989	AC	TAXIWAY	S	0	2,325.00	7/12/2018	29	69
T12AU	01	1/1/2001	AC	TAXIWAY	S	0	2,749.00	7/12/2018	17	66
T13AU	01	1/1/1989	AC	TAXIWAY	S	0	2,992.00	7/12/2018	29	63
TA1AU	01	5/2/2005	AAC	TAXIWAY	Р	0	2,537.00	7/12/2018	13	59
TA1AU	02	9/3/2008	AC	TAXIWAY	Р	0	8,740.00	7/12/2018	10	88
TA2AU	01	5/2/2005	AAC	TAXIWAY	Р	0	3,073.00	7/12/2018		
TA2AU	02	9/3/2008	-	TAXIWAY	P	0	8,595.00		<u> </u>	
TA3AU	01	5/2/2005		TAXIWAY	Р	0	3,403.00		13	
TA3AU TA3AU	02 03	9/3/2007 9/3/2007	AC AC	TAXIWAY TAXIWAY	P P	0	8,813.00 3,190.00	7/12/2018 7/12/2018	11 11	80 88
TA4AU	01	5/2/2005	AAC	TAXIWAY	P	0	3,324.00	7/12/2018	13	
TA4AU	02	9/3/2007	AC	TAXIWAY	P	0	9,028.00	7/12/2018	11	
TA5AU	01	5/2/2005	AC	TAXIWAY	P	0	2,520.00	7/12/2018	13	
TA5AU	02	8/1/2008		TAXIWAY	Р	0	3,188.00	7/12/2018	10	
TA5AU	03	8/1/2008	AAC	TAXIWAY	Р	0	3,975.00	7/12/2018	10	73
TAAAU	01	9/3/2016	AC	TAXIWAY	Р	0	7,284.00	7/12/2018	2	100
TAAU	01	9/3/2008	AC	TAXIWAY	Р	0	56,785.00	7/12/2018		
TAAU	02	9/3/2007	AC	TAXIWAY	Р	0	88,885.00	7/12/2018	11	
TAAU	03	8/1/2008		TAXIWAY	P	0	29,204.00	7/12/2018	10	
TL1AU	01	9/3/2016	AC	TAXIWAY	S	0	9,921.00	7/12/2018	2	100

7/26/2018		Sectio	n Co	ndition Re	eport]	Page 2	of 3
TL2AU	01	9/3/2016	AC	TAXIWAY	S	0	10,673.00	7/12/2018	2	100
TL3AU	01	9/3/2016	AC	TAXIWAY	S	0	15,963.00	7/12/2018	2	100
TNWYLEEAU	01	9/3/2008	AC	TAXIWAY	S	0	3,465.00	7/12/2018	10	75
TSWYLEEAU	01	9/3/2008	AC	TAXIWAY	S	0	3,237.00	7/12/2018	10	94
TWILLAVAU	01	9/3/2008	AC	TAXIWAY	Р	0	3,777.00	7/12/2018	10	89

7/26/2018	Section Condition Report (Summary)	Page 3 of 3
	Pavement Database: ODA_2018 _Final	

Age Category	Average Age at Inspection	Total Area (SqFt)	Number of Sections	Arithmetic Average PCI	Standard Deviation PCI	Weighted Average PCI
00-02	2	43,841.00	4	100.00	0.00	100.00
03-05	3	75,576.00	4	100.00	0.00	100.00
06-10	10	256,949.00	15	79.33	8.39	75.23
11-15	12	628,563.00	12	70.92	11.13	79.32
16-20	17	131,344.00	4	70.25	12.70	57.31
26-30	29	103,303.00	9	63.89	12.90	66.38
41-50	49	9,162.00	1	49.00	0.00	49.00
ALL	14	1,248,738.00	49	76.45	15.61	76.85

Appendix 3 Network Maintenance Report

Network Maintenance Report Aurora State Airport

Network	Branch	Section	Distress	Severity	Action	Work Quantity	Unit	Unit Cost	Work Cost	Section Total Cost
Aurora	A02AU	01	Block Cracking	Medium	Crack Sealing - AC	6,404	Ft	\$1.50	\$9,605	\$9,605
Aurora	A05AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	90	Ft	\$1.50	\$135	
Aurora	A06AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	40	Ft	\$1.50	\$60	\$60
Aurora	A08AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	477	Ft	\$1.50	\$715	\$715
Aurora	A09AU	01	Block Cracking	Medium	Crack Sealing - AC	3,308	Ft	\$1.50	\$4,961	\$4,961
Aurora	AH35AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	758	Ft	\$1.50	\$1,136	\$1,136
Aurora	R17AU	02	Long. & Trans. Cracking	Medium	Crack Sealing - AC	3,960	Ft	\$1.50	\$5,940	\$5,940
Aurora	T01AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	20	Ft	\$1.50	\$30	\$30
Aurora	T02AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	190	Ft	\$1.50	\$285	\$285
Aurora	T07AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	40	Ft	\$1.50	\$60	\$60
Aurora	T08AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	120	Ft	\$1.50	\$180	\$180
Aurora	T09AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	260	Ft	\$1.50	\$390	\$390
Aurora	T11AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	50	Ft	\$1.50	\$75	\$75
Aurora	T12AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	120	Ft	\$1.50	\$180	\$180
Aurora	T13AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	110	Ft	\$1.50	\$165	\$165
Aurora	TA1AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	240	Ft	\$1.50	\$360	\$360
Aurora	TA2AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	130	Ft	\$1.50	\$195	\$195
Aurora	TA3AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	154	Ft	\$1.50	\$230	\$230
Aurora	TA3AU	02	Long. & Trans. Cracking	High	Crack Seal - Wide Cracks	1	Ft	\$30.00	\$30	\$30
Aurora	TA4AU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	250	Ft	\$1.50	\$375	\$375
Aurora	TA4AU	02	Long. & Trans. Cracking	Medium	Crack Sealing - AC	100	Ft	\$1.50	\$150	\$150
Aurora	TA5AU	01	Block Cracking	Medium	Crack Sealing - AC	384	Ft	\$1.50	\$576	\$576
Aurora	TA5AU	02	Long. & Trans. Cracking	Medium	Crack Sealing - AC	110	Ft	\$1.50	\$165	\$165
Aurora	TA5AU	03	Long. & Trans. Cracking	Medium	Crack Sealing - AC	90	Ft	\$1.50	\$135	\$135
Aurora	TAAU	01	Long. & Trans. Cracking	High	Crack Seal - Wide Cracks	3	Ft	\$30.00	\$81	\$81
Aurora	TAAU	02	Long. & Trans. Cracking	Medium	Crack Sealing - AC	1,795	Ft	\$1.50	\$2,692	\$2,692
Aurora	TAAU	03	Long. & Trans. Cracking	Medium	Crack Sealing - AC	1,038	Ft	\$1.50	\$1,558	\$1,558
Aurora	TNWYLEEAU	01	Long. & Trans. Cracking	Medium	Crack Sealing - AC	60	Ft	\$1.50	\$90	
						•			Total	\$30,555

Appendix 4 Re-Inspection Report

Re-Inspection Report

ODA_2018 _Final Generated Date

Page 1 of 49

Generated Date	//26/2018					
Network: Aurora		Name:	Aurora State			
Branch: A01AU	Name:	Apron 01 Aurora	Use:	APRON	Area:	56,334 SqFt
Section: 01	of 1	From: Taxiway 06		To: Tie Dowr	n Apron New	Last Const.: 9/26/2015
Surface: AC	Family: OR-Cat2-AC-2015	Central-AP- Zone:	KUAO	Category: F		Rank: P
Area: 5	6,334 SqFt Length:	343 Ft	Width:	146 Ft		
Slabs:	Slab Length:	Ft Slab	Width:	Ft	Joint Length:	Ft
Shoulder:	Street Type:	Gra	ide: 0		Lanes: 0	
Section Comments:						
Last Insp. Date: 7/12/2 Conditions: PCI: Inspection Comments:	2018 Total 9	Samples: 11	Surveyed	d: 4		
Sample Number: 01	Type: R	Area:	5000.00 SqFt	PCI: 100)	
Sample Comments: <no distress=""></no>						
Sample Number: 02	Type: R	Area:	5000.00 SqFt	PCI: 100)	
Sample Comments:						
<no distress=""></no>						
Sample Number: 06	Type: R	Area:	5000.00 SqFt	PCI: 100)	
Sample Comments:						
<no distress=""></no>						

4600.00 SqFt

PCI: 100

Sample Number: 10
Sample Comments:

Type:

R

Area:

Network: Aurora		Name:	Aurora State			
				A DD ON	1/	00 C40 G F:
Branch: A02AU	Name:	Apron 02 Aurora		APRON Ar)9,649 SqFt
Section: 01	of 1	From: Taxiway 0	9	To: Private Apron		Last Const.: 8/2/2001
Surface: AC	Family: OR-Cat2-Add 2015	C-Central-AP- Zone:	KSPB	Category: E		Rank: P
Area: 109,649	9 SqFt Lengt	h: 523 Ft	Width:	200 Ft		
Slabs:	Slab Length:	Ft SI	lab Width:	Ft	Joint Length:	Ft
Shoulder:	Street Type:	G	rade: 0		Lanes: 0	
Section Comments:						
Last Insp. Date: 7/12/2018	Tota	alSamples: 22	Surveye	d: 5		
Conditions: PCI: 53						
Inspection Comments:						
Sample Number: 03	Type: R	Area:	5000.00 SqFt	PCI: 54		
Sample Comments:						
43 BLOCK CR	L	4000.00 SqFt				
43 BLOCK CR	M	1000.00 SqFt				
57 WEATHERING	L	5000.00 SqFt				
Sample Number: 05	Type: R	Area:	5000.00 SqFt	PCI: 47		
Sample Comments:						
50 PATCHING	L	1050.00 SqFt				
43 BLOCK CR	L	3160.00 SqFt				
43 BLOCK CR	M	790.00 SqFt				
57 WEATHERING	L	5000.00 SqFt				
Sample Number: 11	Type: R	Area:	5000.00 SqFt	PCI: 54		
Sample Comments:						
43 BLOCK CR	L	4000.00 SqFt				
43 BLOCK CR	M	1000.00 SqFt				
57 WEATHERING	L	5000.00 SqFt				
Sample Number: 15	Type: R	Area:	5000.00 SqFt	PCI: 54		
Sample Comments:						
43 BLOCK CR	L	4000.00 SqFt				
43 BLOCK CR	M	1000.00 SqFt				
57 WEATHERING	L	5000.00 SqFt				
Sample Number: 20	Type: R	Area:	5000.00 SqFt	PCI: 54		
Sample Comments:						

4000.00 SqFt 1000.00 SqFt 5000.00 SqFt

L

M L

43

43 57 BLOCK CR

BLOCK CR WEATHERING

Network:	Aurora					Name	e: Aur	ora State						
Branch:	A03AU			Name:	Tie Dov	wn Apro	on 03 Aurora	Use:	APRON		Area:	9,16	2 SqFt	
Section:	01		of 1]	From: T	Γ13AU			To:	End		La	st Const.:	1/1/1969
Surface:	AC	Family:	OR- 201		Central-AP-	Zone	e: KUAO		Cate	gory: F		Ra	nk: S	
Area:		9,162 SqFt		Length:		197 Ft	t	Width:		59 Ft				
Slabs:		Slab Le	ngth:		Ft		Slab Width:		Ft		Joint Leng	th:	F	t
Shoulder:		Street 7	ype:				Grade: 0				Lanes:	0		
Section C	omments:													
Condition	Date: 7/12 as: PCI: a Comments:	49		TotalS	amples: 2	2		Surveye	d: 2					
Condition Inspection	ns: PCI:	49	ne:				3900			PCI: 35				
Condition Inspection Sample N	s: PCI:	49	pe:	TotalS R		rea:	3900	Surveye		PCI: 35				
Condition Inspection Sample N Sample C	n Comments: umber: 01 omments:	49	-	R	Ai	rea:	3900			PCI: 35				
Condition Inspection Sample N Sample C	ns: PCI: n Comments: umber: 01	49	- I			rea: SqFt	3900			PCI: 35				
Condition Inspection Sample N Sample C 50 PA 43 BL	ns: PCI: n Comments: umber: 01 omments:	49	- I I	R	Ai 1360.00	rea: SqFt SqFt	3900			PCI: 35				
Condition Inspection Sample N Sample C 50 PA 43 BL 52 RA	ns: PCI: n Comments: umber: 01 omments: ATCHING LOCK CR	49 : Ty	- I I	R L L	1360.00 2540.00 2540.00	rea: SqFt SqFt				PCI: 35				
Condition Inspection Sample N Sample C 50 PA 43 BL 52 RA Sample N	as: PCI: n Comments: umber: 01 omments: ATCHING LOCK CR AVELING	49 : Ty	I I	R L L M	1360.00 2540.00 2540.00	rea: SqFt SqFt SqFt		0.00 SqFt						
Condition Inspection Sample N Sample C 50 PA 43 BL 52 RA Sample N Sample C	ns: PCI: n Comments: umber: 01 omments: ATCHING LOCK CR AVELING umber: 02	49 : Ty	I I Pe:	R L L M	1360.00 2540.00 2540.00	rea: SqFt SqFt SqFt rea:		0.00 SqFt						

Network: Aurora		Name	e: Aurora State			
Branch: A04AU	Nan	ne: Tie Down Apro	on 04 Aurora Use:	APRON	Area:	87,212 SqFt
Section: 01	of 1	From: A02AU		To: T12AU		Last Const.: 1/1/2008
Surface: AC	Family: OR-Cat2 2015	2-AC-Central-AP- Zone	: KUAO	Category: F		Rank: P
Area: 87,212	SqFt Le	ngth: 520 Ft	Width:	280 Ft		
Slabs:	Slab Length:	Ft	Slab Width:	Ft	Joint Length:	Ft
Shoulder:	Street Type:		Grade: 0		Lanes: 0	
Section Comments:						
Last Insp. Date: 7/12/2018	7	TotalSamples: 19	Surveye	d: 5		
Conditions: PCI: 68						
Inspection Comments:						
Sample Number: 02	Type: F	Area:	3600.00 SqFt	PCI: 62		
Sample Comments:						
43 BLOCK CR	L	2880.00 SqFt				
57 WEATHERING	L	3600.00 SqFt				
Sample Number: 03	Type: I	Area:	5000.00 SqFt	PCI: 62		
Sample Comments:						
43 BLOCK CR	L	4000.00 SqFt				
57 WEATHERING	L	5000.00 SqFt				
Sample Number: 10	Type: F	Area:	5982.00 SqFt	PCI: 67		
Sample Comments:						
57 WEATHERING	L	5982.00 SqFt				
43 BLOCK CR	L	2991.00 SqFt				
Sample Number: 12	Type: I	Area:	5000.00 SqFt	PCI: 76		
Sample Comments:						
48 L & T CR	L	370.00 Ft				
57 WEATHERING	L	5000.00 SqFt				
Sample Number: 18	Type: I	Area:	5000.00 SqFt	PCI: 74		
Sample Comments:						

L L

420.00 Ft

5000.00 SqFt

48

57

L & T CR

WEATHERING

Network: Aurora Name: Aurora State **Branch:** A05AU Name: Apron 05 Aurora Use: APRON Area: 6,184 SqFt 01 **Section:** of 1 From: Taxiway 15 To: Taxiway A3 **Last Const.:** 1/1/1989 Surface: ACFamily: OR-Cat2-AC-Central-AP-Zone: KUAO Category: F Rank: S 2015 6,184 SqFt Length: Width: 48 Ft Area: 150 Ft Ft Slab Width: Slabs: Slab Length: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: TotalSamples:** 1 **Last Insp. Date:** 7/12/2018 Surveyed: 1 **PCI:** 40 **Conditions: Inspection Comments:** Sample Number: 01 R 6184.00 SqFt **PCI:** 40 Type: Area: **Sample Comments:**

52

48

57

RAVELING

WEATHERING

L & T CR

M

M

L

5256.00 SqFt

90.00 Ft

927.00 SqFt

Network: Aurora Name: Aurora State **Branch:** A06AU Name: Apron 06 Aurora Use: APRON Area: 3,790 SqFt 01 **Section:** of 1 From: Taxiway A To: East **Last Const.:** 1/1/2007 Surface: ACFamily: OR-Cat2-AC-Central-AP-Zone: KUAO Category: K Rank: S 2015 79 Ft 3,790 SqFt Length: Width: 48 Ft Area: Ft Slab Width: Ft Slabs: Slab Length: Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 82 **Inspection Comments:** Sample Number: 01 R 3790.00 SqFt PCI: 82 Type: Area: **Sample Comments:**

48

48

57

L & T CR

L & T CR

WEATHERING

L

M

L

10.00 Ft

40.00 Ft

3790.00 SqFt

Network	: Aurora					Nan	ne: Auro	ora State						
Branch:	A07AU	ſ	N	Name:	Apror	07 Aur	ora	Use:	APRON	1	Area:		21,600 SqFt	
Section:	01		of 1	Fre	om:	Taxiwa	y A		To:	East			Last Const.:	1/1/1989
Surface:	PCC	Family:	OR-C -2015	Cat2-PCC-C	entral-A	P Zon	e: KUAO		Cate	egory:	F		Rank: S	
Area:		21,600 SqFt		Length:		450 F	`t	Width:		48 Ft				
Slabs:	78	Slab Le	ength:		20 Ft		Slab Width:		20 Ft		Joir	t Length:	1,662 F	t
Shoulder	:	Street	Гуре:				Grade: 0				Lan	es: 0		
Section (Comments:													
Last Insp	Date: 7/1	2/2018		TotalSan	nples:	3		Surveye	ed: 3					
Condition	ns: PCI:	88												
Inspectio	on Comments	: :												
Sample N	Number: 01	T	ype:	R		Area:	27	.00 Slabs		PCI:	84			
Sample (Comments:													
63 L1	INEAR CR		L		1.00	Slabs								
63 L	INEAR CR		L		4.00	Slabs								
74 JC	DINT SPALL		M		1.00	Slabs								
Sample N	Number: 02	T ₂	pe:	R	1	Area:	21	.00 Slabs		PCI:	84			
Sample (Comments:													
63 L	INEAR CR		L		2.00	Slabs								
63 Ll	INEAR CR		L		1.00	Slabs								
74 JC	DINT SPALL		M		2.00	Slabs								
Sample N	Number: 03	T	ype:	R		Area:	24	.00 Slabs		PCI:	96			
Samnle (Comments:													

LINEAR CR L 1.00 Slabs

Netw	ork: Auro	ra					Nai	ne: Auro	ora State						
Bran	ch: A08A	U			Name:	Apron	08 Au	rora	Use:	APRON		Area:	22	2,503 SqFt	
Secti	on: 01		of	1		From:	Taxiwa	ny A		To:	East			Last Const.:	1/1/1989
Surfa	ace: AC		Family:	OR- 2015		-Central-AP	- Zoi	ne: KUAO		Categ	ory: F			Rank: S	
Area	:	22,50	3 SqFt		Length:		480	Ft	Width:		48 Ft				
Slabs	s:		Slab Leng	gth:		Ft		Slab Width:		Ft		Joint Len	gth:	F	t
Shou	lder:		Street Ty	pe:				Grade: 0				Lanes:	0		
Secti	on Comments:														
Last	Insp. Date: 7	/12/2018	3		Totals	Samples:	5		Surveye	ed: 3					
Conc	litions: PCI	: 70													
Insp	ection Comme	ıts:													
Sam	ole Number:	02	Турс	e:	R		Area:	4800	.00 SqFt	F	PCI: 74				
Sam	ole Comments:								-						
48	L & T CR			L	<u>.</u>	130.00	Ft								
48	L & T CR			N		100.00									
57	WEATHERI	NG		L	_	4800.00	SqFt								
Sam	ple Number:	03	Type	e:	R	A	Area:	4800	.00 SqFt	F	PCI: 75				
Sam	ple Comments:														
48	L & T CR			L		75.00	Ft								
48	L & T CR			N	Л	75.00	Ft								
45	DEPRESSIC			L			SqFt								
57	WEATHERI	NG		L	,	4800.00	SqFt								
Sam	ple Number:	04	Тур	e:	R	A	Area:	4800	.00 SqFt	F	PCI: 60				
Sam	ole Comments:														
48	L & T CR			L		200.00	Ft								
48	L & T CR			N	Л	130.00	Ft								
45	DEPRESSIC	N		L			SqFt								
57	WEATHERI			L		4800.00	-								
41	ALLIGATO:	R CR		L		45.00	SqFt								

Netw	ork:	Aurora						Nam	e: Auro	ra State						
Bran	ich:	A09AU				Name:	Apror	o 09 Auro	ora	Use:	APRON		Area:		44,087 SqFt	
Section	on:	01		of	f 3		From:	Taxiway	7 10		To: Ea	st			Last Const.:	1/1/1989
Surfa	ace:	AC		Family:	OR- 201		-Central-AP	- Zone	: KUAO		Categor	y: F			Rank: S	
Area	:		21,70	05 SqFt		Length	:	231 Ft	t	Width:	132	Ft				
Slabs	s:			Slab Len	gth:		Ft		Slab Width:		Ft		Joint Len	gth:	Ft	t
Shou	lder:			Street Ty	ype:				Grade: 0				Lanes:	0		
Section	on Co	omments:														
Last	Insp.	Date: 7/1	2/2018	3		Total	Samples:	5		Surveye	ed: 3					
Cond	litions	s: PCI:	49													
Inspe	ection	Comments	:													
Samp	ple Nu	umber: 01		Тур	e:	R		Area:	5000	.00 SqFt	PC	I: 49				
Samp	ple Co	omments:														
43	BL	OCK CR]	L	2500.00	SqFt								
43	BL	OCK CR			1	M	2500.00	SqFt								
57	WE	EATHERING	j]	L	5000.00	SqFt								
Samp	ple Nu	umber: 02		Тур	e:	R		Area:	5000	.00 SqFt	PC	I: 49				
Samp	ple Co	omments:														
43	BL	OCK CR]	L	2500.00	SqFt								
43		OCK CR			I	M	2500.00									
57	WE	EATHERING	3]	L	5000.00	SqFt								
Samp	ple Nu	umber: 05		Тур	e:	R		Area:	4385	.00 SqFt	PC	I: 49				
Samp	ple Co	omments:														
43	BL	OCK CR]	L	2193.00	SqFt								
43	BL	OCK CR			1	M	2192.00	SqFt								
57	WF	EATHERING	j]	L	4385.00	SqFt								

Network: Aurora			Nan	ne: Auro	ora State						
Branch: A09AU	1	Name:	Apron 09 Aur	ora	Use:	APRON	A	rea:	4	44,087 SqFt	
Section: 02	of 3	Fron	n: Taxiwa	y 10		To: South				Last Cons	6/1/2010
Surface: AC	Family: OR-C 2015	Cat2-AC-Centr	ral-AP- Zon	e: KUAO		Category: 1	7			Rank: S	
Area: 13	,596 SqFt	Length:	103 F	⁷ t	Width:	132 Ft					
Slabs:	Slab Length:		Ft	Slab Width:		Ft		Joint Len	gth:		Ft
Shoulder:	Street Type:			Grade: 0				Lanes:	0		
Section Comments:											
Last Insp. Date: 7/12/20	018	TotalSamp	oles: 3		Surveye	d: 2					
_		TotalSamp	des: 3		Surveye	d: 2					
Conditions: PCI: 7		TotalSamp	lles: 3		Surveye	d: 2					
Conditions: PCI: 7. Inspection Comments:		TotalSamp	des: 3	5150	Surveye	d: 2 PCI:	90				
Conditions: PCI: 7. Inspection Comments: Sample Number: 02	5			5150			90				
Conditions: PCI: 7. Inspection Comments: Sample Number: 02 Sample Comments:	5	R		5150			90				
Inspection Comments: Sample Number: 02 Sample Comments:	Туре:	R	Area:	5150			90				
Conditions: PCI: 7. Inspection Comments: Sample Number: 02 Sample Comments: 48 L & T CR 57 WEATHERING	5 Type:	R	Area: 40.00 Ft								
Conditions: PCI: 7. Inspection Comments: Sample Number: 02 Sample Comments: 48 L & T CR 57 WEATHERING Sample Number: 03	Type:	R 51	Area: 40.00 Ft 50.00 SqFt		.00 SqFt	PCI:					
Conditions: PCI: 7. Inspection Comments: Sample Number: 02 Sample Comments: 48 L & T CR	Type:	R 51	Area: 40.00 Ft 50.00 SqFt		.00 SqFt	PCI:					

Network: Aur	rora				Name:	Auro	ora State						
Branch: A09	9 AU	N	lame:	Apron	09 Aurora	Į	Use:	APRON		Area:	44	4,087 SqFt	
Section: 03		of 3	Fr	rom: I	Paved Infil	11		To:	-			Last Const	.: 6/1/2010
Surface: AC	Family:	OR-C 2015	at2-AC-Ce	entral-AP-	Zone:	KUAO		Cate	gory: F			Rank: S	
Area:	8,786 SqFt]	Length:		145 Ft		Width:		68 Ft				
Slabs:	Slab L	ength:		Ft	SI	lab Width:		Ft		Joint Leng	th:		Ft
Shoulder:	Street '	Туре:			G	rade: 0				Lanes:	0		
Section Comment	s:												
Conditions: PC	CI: 88		TotalSa	mples: 2	2		Surveye	d : 2					
Conditions: PC Inspection Comm	CI: 88 ents:	vpe:	TotalSar R		rea:	4393	-		PCI: 90				
Conditions: PC Inspection Commo Sample Number:	CI: 88 ents: 01 T	ype:				4393	Surveye		PCI : 90				
Conditions: PC Inspection Common Sample Number: Sample Comment	CI: 88 ents: 01 T	ype:			rea:	4393	-		PCI : 90				
Conditions: PC Inspection Common Sample Number: Sample Comment 48 L & T CR	CI: 88 ents: 01 T			A	rea: Ft	4393	-		PCI: 90				
Conditions: PC Inspection Common Sample Number: Sample Comment 48 L & T CR 57 WEATHER	CI: 88 ents: 01 T s:	L		25.00 4393.00	rea: Ft		-		PCI: 90 PCI: 85				
Sample Number: Sample Comment 48 L & T CR	CI: 88 ents: 01 T s: RING 02 T	L L	R	25.00 4393.00	rea: Ft SqFt		.00 SqFt						
Conditions: PC Inspection Common Sample Number: Sample Comment 48 L & T CR 57 WEATHER Sample Number:	CI: 88 ents: 01	L L	R	25.00 4393.00	rea: Ft SqFt rea:		.00 SqFt						
Conditions: PC Inspection Comm Sample Number: Sample Comment 48 L&TCR 57 WEATHER Sample Number: Sample Comment	CI: 88 ents: 01	L L ype:	R	25.00 4393.00 A	rea: Ft SqFt rea: SqFt Ft		.00 SqFt						

Network: Aurora		Name:	Aurora State			
Branch: AH35AU	Name:	Hold Apron 35 Au	rora Use:	APRON	Area:	19,308 SqFt
Section: 01	of 1	From: Taxiway A		To: END		Last Const.: 8/1/2008
Surface: AC	Family: OR-Cat2-AC-2015	Central-AP- Zone:	KUAO	Category: F		Rank: P
Area: 19,3	08 SqFt Length:	225 Ft	Width:	80 Ft		
Slabs:	Slab Length:	Ft Sla	b Width:	Ft	Joint Length:	Ft
Shoulder:	Street Type:	Gr	ade: 0		Lanes: 0	
Section Comments:						
Last Insp. Date: 7/12/201	8 Total	Samples: 4	Surveye	d: 3		
Conditions: PCI: 71						
Inspection Comments:						
Sample Number: 01	Type: R	Area:	3723.00 SqFt	PCI: 78		
Sample Comments:						
48 L & T CR	L	100.00 Ft				
48 L & T CR 48 L & T CR	L M	100.00 Ft 90.00 Ft				
48 L & T CR			5964.00 SqFt	PCI: 73		
48 L & T CR Sample Number: 02	M	90.00 Ft	5964.00 SqFt	PCI: 73		
48 L & T CR Sample Number: 02 Sample Comments:	M	90.00 Ft	5964.00 SqFt	PCI: 73		
48 L & T CR Sample Number: 02 Sample Comments: 48 L & T CR	Type: R	90.00 Ft Area:	5964.00 SqFt	PCI: 73		
48 L&TCR Sample Number: 02 Sample Comments: 48 L&TCR 48 L&TCR	Type: R	90.00 Ft Area: 300.00 Ft	5964.00 SqFt 5989.00 SqFt	PCI: 73		
48 L & T CR Sample Number: 02 Sample Comments: 48 L & T CR 48 L & T CR 548 L & T CR 550 Sample Number: 03	Type: R L M	90.00 Ft Area: 300.00 Ft 225.00 Ft				
48 L & T CR Sample Number: 02 Sample Comments: 48 L & T CR	Type: R L M	90.00 Ft Area: 300.00 Ft 225.00 Ft				
48 L & T CR Sample Number: 02 Sample Comments: 48 L & T CR 48 L & T CR 48 L & T CR Sample Number: 03 Sample Comments:	Type: R L M Type: R	90.00 Ft Area: 300.00 Ft 225.00 Ft Area:				

Network: A	urora			Namo	e: Aurora State			
Branch: R	17AU		Name:	Runway 17/35	Aurora Use:	RUNWAY	Area: 5	500,000 SqFt
Section: 01		of 2	,	From: Runway	17 End	To: Section 02		Last Const.: 5/2/2005
Surface: AC		• • • • • • • • • • • • • • • • • • • •	R-Cat2-AC 015	-Central-RW- Zone	: KUAO	Category: F		Rank: P
Area:	410,00	0 SqFt	Length	4,100 Ft	Width:	100 Ft		
Slabs:		Slab Length	:	Ft	Slab Width:	Ft	Joint Length:	Ft
Shoulder:		Street Type:			Grade: 0		Lanes: 0	
Section Comme	nts:							
Last Insp. Date:	7/12/2018		Total	Samples: 82	Survey	r ed: 6		
Conditions: I	PCI: 83							
Inspection Com	ments:							
Sample Number	: 01	Type:	R	Area:	5000.00 SqFt	PCI: 81		
Sample Comme	nts:							
48 L & T CF	{		L	350.00 Ft				
Sample Number	: 21	Type:	R	Area:	5000.00 SqFt	PCI: 81		
Sample Comme	nts:							
48 L & T CF	{		L	350.00 Ft				
Sample Number	: 38	Type:	R	Area:	5000.00 SqFt	PCI: 82		
Sample Comme	nts:							
48 L & T CF	}		L	330.00 Ft				
Sample Number	: 51	Type:	R	Area:	5000.00 SqFt	PCI: 85		
Sample Comme	nts:							
48 L & T CF	t		L	260.00 Ft				
Sample Number	: 68	Type:	R	Area:	5000.00 SqFt	PCI: 82		
Sample Comme	nts:							
48 L & T CF	}		L	320.00 Ft				
Sample Number		Type:	R	Area:	5000.00 SqFt	PCI: 84		
Sample Comme		• •			•			

L

280.00 Ft

48

L & T CR

Network: Aurora		Name:	Aurora State		
Branch: R17AU	Name:	Runway 17/35 A	urora Use:	RUNWAY Area:	500,000 SqFt
Section: 02	of 2	rom: Section 01		To: Runway 35 End	Last Const.: 5/1/2005
Surface: AAC	Family: OR-Cat2-AAC-RW-2015	Central- Zone:	KUAO	Category: F	Rank: P
Area: 90,000	SqFt Length:	900 Ft	Width:	100 Ft	
Slabs:	Slab Length:	Ft S	lab Width:	Ft Joi r	nt Length: Ft
Shoulder:	Street Type:	G	Grade: 0	Lan	nes: 0
Section Comments:					
Last Insp. Date: 7/12/2018	TotalSa	mples: 18	Surveyed	l: 5	
Conditions: PCI: 72					
Inspection Comments:					
Sample Number: 01	Type: R	Area:	5000.00 SqFt	PCI: 78	
Sample Comments:					
48 L & T CR	L	300.00 Ft			
48 L & T CR	M	50.00 Ft			
Sample Number: 06	Type: R	Area:	5000.00 SqFt	PCI: 65	
Sample Comments:					
48 L & T CR	M	450.00 Ft			
Sample Number: 10	Type: R	Area:	5000.00 SqFt	PCI: 72	
Sample Comments:					
48 L & T CR	L	200.00 Ft			
48 L & T CR	M	200.00 Ft			
Sample Number: 14	Type: R	Area:	5000.00 SqFt	PCI: 69	
Sample Comments:					
48 L & T CR	L	200.00 Ft			
48 L & T CR	M	250.00 Ft			
Sample Number: 17	Type: R	Area:	5000.00 SqFt	PCI: 76	
Sample Comments:					

L & T CR L & T CR

48

48

L M

200.00 Ft

150.00 Ft

Network	: Aurora			N	ame: Auro	ora State					
Branch:	T01AU		Name:	Taxiway 01	Aurora	Use:	TAXIWA	Y	Area:	9,478 SqFt	
Section:	01	0	f 1	From: Tie I	Down Apron New		To:	Hangars		Last Const.:	8/1/2001
Surface:	AC	Family:	OR-Cat2-A 2015	C-Central-TW- Z	one: KUAO		Catego	ory: F		Rank: S	
Area:		9,478 SqFt	Lengt	:h: 380	0 Ft	Width:		25 Ft			
Slabs:		Slab Len	ngth:	Ft	Slab Width:		Ft		Joint Length	ı: I	t
Shoulder	:	Street T	ype:		Grade: 0				Lanes: 0)	
Section C	Comments:										
	7/10										
Last Insp	Date: 7/12	2/2018	Tot	alSamples: 2		Surveye	ed: 2				
Condition			Tot	alSamples: 2		Surveye	ed: 2				
Condition		88	Tot	alSamples: 2		Surveye	ed: 2				
Condition Inspection	ns: PCI:	88		alSamples: 2 Area:	4362	Surveye		CI: 89			
Condition Inspection Sample N	ns: PCI: on Comments:	88			4362			CI: 89			
Condition Inspection Sample N Sample C	ns: PCI: on Comments: Number: 01	88			4362			CI: 89			
Condition Inspection Sample N Sample C	ns: PCI: on Comments: Number: 01 Comments:	88 : Typ	pe: R	Area:				CI: 89			
Condition Inspection Sample N Sample C 48 L 57 W	ns: PCI: on Comments: Number: 01 Comments: & T CR	88 : Тур	pe: R L L	Area: 40.00 Ft	't		P	CI: 89			
Condition Inspection Sample N Sample C 48 L 57 W Sample N	ns: PCI: on Comments: Number: 01 Comments: & T CR /EATHERING	88 : Тур	pe: R L L	Area: 40.00 Ft 4362.00 SqF	't	2.00 SqFt	P				
Condition Inspectio Sample N Sample C 48 L 57 W Sample N Sample C	ns: PCI: on Comments: Number: 01 Comments: & T CR /EATHERING	88 : Тур	pe: R L L	Area: 40.00 Ft 4362.00 SqF	't	2.00 SqFt	P				

Network	: Aurora					Name:	: Auro	ora State				
Branch:	T02AU		ľ	Name:	Taxiwa	y 02 Aur	ora	Use:	TAXIWAY	Area:	9,468 SqFt	
Section:	01	0	f 1	F	rom:	Γie Down	Apron New		To: Hangars		Last Const.:	8/1/2001
Surface:	AC	Family:	OR-0 2015		entral-TW-	Zone:	KUAO		Category: F		Rank: S	
Area:		9,468 SqFt		Length:		378 Ft		Width:	25 Ft			
Slabs:		Slab Len	igth:		Ft	S	lab Width:		Ft	Joint Length:	F	t
Shoulder	r:	Street T	ype:			(Grade: 0			Lanes: 0		
Section C	Comments:											
	ons: PCI: on Comments:	74 :										
Inspectio Sample N	on Comments Number: 01		pe:	R	A	rea:	4378	.00 SqFt	PCI: 71			
Inspectio Sample N Sample C	Number: 01 Comments:	:					4378	.00 SqFt	PCI: 71			
Inspection Sample M Sample C	on Comments: Number: 01 Comments: & T CR	:	L		40.00	Ft	4378	.00 SqFt	PCI: 71			
Sample N Sample C 48 L 48 L	Number: 01 Comments: & T CR & T CR	: Туј	L M	[40.00 120.00	Ft Ft	4378	.00 SqFt	PCI: 71			
Sample N Sample C 48 L 48 L 57 W	on Comments: Number: 01 Comments: & T CR	: Туј	L	I	40.00	Ft Ft SqFt	4378	.00 SqFt	PCI: 71			
Sample N Sample C 48 L 48 L 57 W 50 P	on Comments: Number: 01 Comments: & T CR & T CR VEATHERING	: Тур	L M L L	I	40.00 120.00 4378.00 25.00	Ft Ft SqFt		.00 SqFt	PCI: 71			
Sample N Sample C 48 L 48 L 57 W 50 P2 Sample N	on Comments: Number: 01 Comments: & T CR & T CR VEATHERING	: Туј	L M L L	I	40.00 120.00 4378.00 25.00	Ft Ft SqFt SqFt						
Inspection Sample N Sample C 48 L 48 L 57 W 50 P Sample N Sample C	on Comments: Number: 01 Comments: & T CR & T CR VEATHERING ATCHING Number: 02	: Туј	L M L L	R	40.00 120.00 4378.00 25.00	Ft Ft SqFt SqFt rea:						
Sample C 48	Number: 01 Comments: & T CR & T CR VEATHERING ATCHING Number: 02 Comments:	: Туј	L M L L	R	40.00 120.00 4378.00 25.00	Ft Ft SqFt SqFt rea:						

Aurora State Network: Aurora Name: **Branch:** T03AU Name: Taxiway 03 Aurora Use: TAXIWAYArea: 3,684 SqFt 01 **Section:** of 1 From: Taxiway A To: Apron 01 **Last Const.:** 9/26/2015 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 3,684 SqFt Length: Width: 35 Ft Area: 83 Ft Ft Slab Width: Slabs: Slab Length: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: 0 **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 100 **Conditions: Inspection Comments:** Sample Number: 01 R 3684.00 SqFt **PCI:** 100 Type: Area:

Sample Comments:

Aurora State Network: Aurora Name: **Branch:** T04AU Name: Taxiway 04 Aurora Use: TAXIWAYArea: 3,880 SqFt 01 **Section:** of 1 From: Taxiway A To: Apron 01 **Last Const.:** 9/26/2015 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 3,880 SqFt Length: Width: 40 Ft Area: 75 Ft Ft Slab Width: Ft Slabs: Slab Length: Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: 0 **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 100 **Conditions: Inspection Comments:** Sample Number: 01 R 3880.00 SqFt **PCI:** 100 Type: Area:

Sample Comments:

Network: Aurora Name: Aurora State **Branch:** T05AU Name: Taxiway 05 Aurora Use: TAXIWAY Area: 11,678 SqFt **Section:** 01 of 1 From: Apron 01 To: Apron 02 **Last Const.:** 9/26/2015 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 11,678 SqFt Length: Width: 35 Ft Area: 228 Ft Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 2 Surveyed: 2 **Conditions: PCI:** 100 **Inspection Comments:** Sample Number: 01 R **PCI:** 100 Type: 5236.00 SqFt Area: **Sample Comments:** <No Distress>

6441.00 SqFt

PCI: 100

Sample Number: 02 **Sample Comments:**

Type:

R

Area:

Network: Aurora Name: Aurora State Branch: T06AU Name: Taxiway 06 Aurora Use: TAXIWAY Area: 3,128 SqFt 01 TAAU-01 To: A02AU-01 **Section:** of 1 From: Last Const.: 9/3/2008 OR-Cat2-AC-Central-TW- Zone: Surface: ACFamily: KUAO Category: F Rank: S 2015 48 Ft 3,128 SqFt Length: Width: 36 Ft Area: Slab Length: Ft Slab Width: Slabs: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 80 **Conditions: Inspection Comments:** Sample Number: 01 R 3128.00 SqFt **PCI:** 80 Type: Area: **Sample Comments:**

48

57

L & T CR

WEATHERING

L

L

160.00 Ft

3128.00 SqFt

Network: Aurora Name: Aurora State Branch: T07AU Name: Taxiway 07 Aurora Use: TAXIWAY 3,953 SqFt Area: Section: 01 of 1 From: TAAU To: Private Apron **Last Const.:** 8/1/2008 OR-Cat2-AAC-Central-Surface: AAC Family: Zone: KUAO Category: F Rank: S TW-2015 Length: Width: 60 Ft 3,953 SqFt 48 Ft Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 79 **Inspection Comments:** Sample Number: 01 R 3953.00 SqFt **PCI:** 79 Type: Area: **Sample Comments:** 48 L & T CR L 40.00 Ft

L & T CR

WEATHERING

M

L

40.00 Ft

3953.00 SqFt

48

Network: Aurora Name: Aurora State Branch: T08AU Name: Taxiway 08 Aurora Use: TAXIWAY Area: 4,516 SqFt **Section:** 01 of 1 From: Taxiway 05 To: Apron 05 **Last Const.:** 1/1/1989 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 Length: Width: 25 Ft Area: 4,516 SqFt 174 Ft Ft Slab Width: Slabs: Slab Length: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 64 **Inspection Comments:** Sample Number: 01 R 4516.00 SqFt **PCI:** 64 Type: Area: **Sample Comments:**

41 ALLIGATOR CR L 80.00 SqFt 48 L & T CR M 120.00 Ft 48 L & T CR L 200.00 Ft

Network:	Aurora			Na	ame: Aur	ora State				
Branch:	T09AU		Name:	Taxiway 09	Aurora	Use:	TAXIWAY	Area:	12,198 SqFt	
Section:	01	of 1		From: Apror	n 05		To: End		Last Const.:	1/1/1989
Surface:	AC		R-Cat2-AC 015	-Central-TW- Zo	one: KUAO		Category: F		Rank: S	
Area:	12,	198 SqFt	Length	: 464	Ft	Width:	26 Ft			
Slabs:		Slab Length	:	Ft	Slab Width:		Ft	Joint Leng	gth: F	t
Shoulder:		Street Type:			Grade: 0			Lanes:	0	
Last Insn.	Date: 7/12/20	18	Total	Samples: 2		Surveye	d: 2			
_	Date: 7/12/20 s: PCI: 71		Total	Samples: 2		Surveye	d: 2			
Condition	s: PCI: 71			Samples: 2						
Condition: Inspection	s: PCI : 71		Total	Samples: 2	686-	Surveye	d: 2 PCI: 6	58		
Conditions Inspection Sample Nu	s: PCI: 71 n Comments: umber: 01				6864			58		
Conditions Inspection Sample Nu Sample Co	s: PCI: 71 n Comments: umber: 01				6864			58		
Condition: Inspection Sample Nu Sample Co 48 L & 48 L &	s: PCI: 71 n Comments: umber: 01 omments: & T CR & T CR		R	Area: 240.00 Ft 260.00 Ft				58		
Condition: Inspection Sample No Sample Co 48 L & 48 L &	s: PCI: 71 n Comments: umber: 01 omments:		R L	Area: 240.00 Ft				58		
Condition: Inspection Sample No Sample Co 48 L & 48 L & 57 WE	s: PCI: 71 n Comments: umber: 01 omments: & T CR & T CR		R L M	Area: 240.00 Ft 260.00 Ft	;					
Condition: Inspection Sample No Sample Co 48 L & 48 L & 57 WE	s: PCI: 71 n Comments: umber: 01 omments: & T CR & T CR EATHERING umber: 02	Туре:	R L M L	Area: 240.00 Ft 260.00 Ft 6864.00 SqFt	;	1.00 SqFt	PCI: 6			
Conditions Inspection Sample No Sample Co 48 L & 48 L & 57 WE Sample No Sample Co	s: PCI: 71 n Comments: umber: 01 omments: & T CR & T CR EATHERING umber: 02	Туре:	R L M L	Area: 240.00 Ft 260.00 Ft 6864.00 SqFt	;	1.00 SqFt	PCI: 6			

Network:	Aurora			Name	: Aurora State			
Branch:	T10AU		Name:	Taxiway 10 Au	rora Use:	TAXIWAY	Area:	9,280 SqFt
Section: (01	of	1	From: Apron 05		To: End		Last Const.: 1/1/1989
Surface: A	AC	Family:	OR-Cat2-AC- 2015	Central-TW- Zone:	KUAO	Category: F		Rank: S
Area:		9,280 SqFt	Length:	464 Ft	Width:	20 Ft		
Slabs:		Slab Leng	gth:	Ft S	Slab Width:	Ft	Joint Length:	Ft
Shoulder:		Street Ty	pe:		Grade: 0		Lanes: 0	
Section Con	nments:							
Last Insp. D	Date: 7/12	/2018	Totals	Samples: 2	Survey	ed: 2		
Conditions:	PCI:	61						
Inspection (Comments:							
Sample Nur	mber: 01	Туре	e: R	Area:	5280.00 SqFt	PCI: 64		
Sample Con	nments:							
52 RAV	ELING		L	5280.00 SqFt				
48 L&	T CR		L	440.00 Ft				
57 WEA	ATHERING	ł	L	5280.00 SqFt				
Sample Nur	mber: 02	Турс	e: R	Area:	4000.00 SqFt	PCI: 59		
Sample Con	nments:							
57 WEA	ATHERING	ł	L	4000.00 SqFt				
	T CR		L	270.00 Ft				
	CHING		L	90.00 SqFt				
52 RAV	ELING		L	4000.00 SqFt				

Network:	Aurora				Name:	Aurora	State			
Branch:	T11AU		Name:	Taxiway	/ 11 Aurora		Use:	TAXIWAY	Area:	2,325 SqFt
Section: 0)1	0	f 1	From: A	pron 05			To: End		Last Const.: 1/1/1989
Surface: A	AC	Family:	OR-Cat2-AC 2015	-Central-TW-	Zone:	KUAO		Category: F		Rank: S
Area:	2	2,325 SqFt	Length	:	85 Ft	v	Vidth:	25 Ft		
Slabs:		Slab Len	igth:	Ft	Slab	Width:		Ft	Joint Length:	Ft
Shoulder:	oulder: Street Type:			Grade: 0					Lanes: 0	
Section Com	nments:									
Last Insp. D	Pate: 7/12/2	018	Total	Samples: 1			Surveye	d: 1		
Conditions:	PCI: 6	i9								
nspection C	Comments:									
Sample Num	nber: 01	Тур	oe: R	Ar	ea:	2325.0	0 SqFt	PCI:	59	
Sample Com	nments:									
18 L&7	ΓCR		L	60.00	Ft					
18 L&7	ΓCR		M	50.00	Ft					
0 PATO	CHING		L	80.00	SqFt					

Network: Aurora Name: Aurora State Branch: T12AU Name: Taxiway 12 Aurora Use: TAXIWAY Area: 2,749 SqFt **Section:** 01 of 1 From: To: End **Last Const.:** 1/1/2001 Taxiway A Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 2,749 SqFt Length: Width: 35 Ft Area: 48 Ft Ft Slab Width: Slabs: Slab Length: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 66 **Inspection Comments:** Sample Number: 01 R 2749.00 SqFt **PCI:** 66 Type: Area: **Sample Comments:**

48

48

57

L & T CR

L & T CR

WEATHERING

L

M

L

250.00 Ft

120.00 Ft

2749.00 SqFt

Network: Aurora Name: Aurora State **Branch:** T13AU Name: Taxiway 13 Aurora Use: TAXIWAY Area: 2,992 SqFt **Section:** 01 of 1 From: To: End **Last Const.:** 1/1/1989 Taxiway A Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 2,992 SqFt Length: Width: 48 Ft 40 Ft Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: TotalSamples:** 1 **Last Insp. Date:** 7/12/2018 Surveyed: 1 **Conditions: PCI:** 63 **Inspection Comments:** Sample Number: 01 R 2992.00 SqFt **PCI:** 63 Type: Area: **Sample Comments:** L & T CR L 120.00 Ft 48

48

57

57

L & T CR

WEATHERING

WEATHERING

M

L

M

110.00 Ft

2543.00 SqFt

Network: Aurora Name: Aurora State **Branch:** TA1AU Name: Taxiway A1 Aurora Use: TAXIWAY Area: 11,277 SqFt 01 To: TA1AU-01 **Section:** of 2 From: Runway 17 End **Last Const.:** 5/2/2005 OR-Cat2-AAC-Central-TW-2015 Surface: AAC Family: Zone: KUAO Category: F Rank: P Length: 50 Ft Width: 40 Ft Area: 2,537 SqFt Ft Slab Width: Ft Slabs: Slab Length: Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 59 **Conditions: Inspection Comments:** Sample Number: 01 R 2537.00 SqFt **PCI:** 59 Type: Area: **Sample Comments:**

48

57

L & T CR

WEATHERING

M

L

240.00 Ft

Network: Aurora			Nai	me: Aur	ora State						
Branch: TA1AU		Name:	Taxiway A1	Aurora	Use:	TAXIW	AY	Area:	1	1,277 SqFt	
Section: 02	of 2]	From: TA1A	U-01		To:	TAAU-01			Last Const.	: 9/3/2008
Surface: AC	Family: OR- 201		Central-TW- Zoi	ne: KUAO		Cate	gory: F			Rank: P	
Area:	8,740 SqFt	Length:	183	Ft	Width:		40 Ft				
Slabs:	Slab Length:		Ft	Slab Width:		Ft		Joint Len	gth:		Ft
Shoulder:	Street Type:			Grade: 0				Lanes:	0		
Section Comments:											
Last Insp. Date: 7/12	/2018	TotalS	Samples: 2		Surveye	ed: 2					
_	/2018 88	TotalS	Samples: 2		Surveye	ed: 2					
Conditions: PCI:	88	TotalS	samples: 2		Surveye	ed: 2					
Inspection Comments:	88	TotalS	Samples: 2	4574			PCI: 89				
Conditions: PCI: Inspection Comments: Sample Number: 01	88			4574	Surveye		PCI: 89				
Conditions: PCI: Inspection Comments: Sample Number: 01 Sample Comments:	Type:	R	Area:	4574			PCI: 89				
Conditions: PCI:	Type:			4574			PCI: 89				
Conditions: PCI: Inspection Comments: Sample Number: 01 Sample Comments: 48 L & T CR 57 WEATHERING	Type:	R	Area: 60.00 Ft				PCI: 89				
Conditions: PCI: Inspection Comments: Sample Number: 01 Sample Comments: 48 L&TCR 57 WEATHERING Sample Number: 02	Type:	R	Area: 60.00 Ft 4574.00 SqFt		1.00 SqFt						
Conditions: PCI: Inspection Comments: Sample Number: 01 Sample Comments: 48 L&TCR	Type: Type:	R	Area: 60.00 Ft 4574.00 SqFt		1.00 SqFt						

Network: Aurora Name: Aurora State **Branch:** TA2AU Name: Taxiway A2 Aurora Use: TAXIWAY Area: 11,668 SqFt 01 To: TA2AU-02 **Section:** of 2 From: **Runway 17/35 Last Const.:** 5/2/2005 OR-Cat2-AAC-Central-Surface: AAC Family: Zone: KUAO Category: F Rank: P TW-2015 Length: Width: 40 Ft 3,073 SqFt 50 Ft Area: Ft Ft Slabs: Slab Length: Slab Width: Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: TotalSamples:** 1 **Last Insp. Date:** 7/12/2018 Surveyed: 1 **Conditions: PCI:** 67 **Inspection Comments:** Sample Number: 01 R 3073.00 SqFt **PCI:** 67 Type: Area: **Sample Comments:**

48

48

57

L & T CR

L & T CR

WEATHERING

L

M

L

130.00 Ft

130.00 Ft

Network:	Aurora				ľ	Name:	Auro	ra State								
Branch:	TA2AU		N	Name:	Taxiway A	A2 Aurora		Use:	TAXIW	AY	Are	a:	1	11,668 Sc	qFt	
Section: (02	of	2	F	rom: TA2	2AU-01			To:	TAAU-	01			Last C	onst.:	9/3/2008
Surface: A	AC	•	OR-C 2015		entral-TW- 2	Zone:	KUAO		Cate	gory: F				Rank:	P	
Area:	8	,595 SqFt		Length:	18	33 Ft		Width:		40 Ft						
Slabs:		Slab Leng	gth:		Ft	Slab	Width:		Ft			Joint Len	igth:		Ft	
Shoulder:		Street Typ	pe:			Grad	le: 0					Lanes:	0			
Section Con																
Section Con	nments:															
		018		TotalSa	mples: 2			Surveve	ed: 2							
Last Insp. D	Date: 7/12/20			TotalSa	mples: 2			Surveye	ed: 2							
Last Insp. D	PCI: 8			TotalSa	mples: 2			Surveye	ed: 2							
	PCI: 8		e:	TotalSa	mples: 2	:	4595.	Surveye		PCI : 9	0					
Last Insp. D Conditions: Inspection C Sample Num	PCI: 8 Comments:	9	e:			:	4595.			PCI: 9	0					
Last Insp. D Conditions: Inspection (Sample Nur Sample Con	PCI: 8 Comments: nber: 01 nments:	9			Area	:	4595.			PCI: 9	0					
Last Insp. D Conditions: Inspection C Sample Nur Sample Con	PCI: 8 Comments: nber: 01 nments:	9	e: L L	R			4595.			PCI: 9	0					
Last Insp. D Conditions: Inspection C Sample Num Sample Con 48 L&7 57 WEA	PCI: 8 Comments: nber: 01 nments: T CR ATHERING	9	L L	R	Area	Ft				PCI: 9						
Last Insp. D Conditions: Inspection C Sample Num Sample Com 48 L& 2 57 WEA Sample Num	PCI: 8 Comments: nber: 01 nments: T CR ATHERING nber: 02	Туре	L L	R	30.00 Ft 4595.00 Sql	Ft		00 SqFt								
Last Insp. D Conditions: Inspection O Sample Num Sample Con 48 L & 57 WEA Sample Num Sample Con	PCI: 8 Comments: nber: 01 nments: T CR ATHERING nber: 02	Туре	L L	R	30.00 Ft 4595.00 Sql	Ft		00 SqFt								

Network: Aurora Name: Aurora State **Branch:** TA3AU Name: Taxiway A3 Aurora Use: TAXIWAY Area: 15,406 SqFt 01 **To:** TA3AU-02 **Section:** of 3 From: **Runway 17/35 Last Const.:** 5/2/2005 OR-Cat2-AAC-Central-Surface: AAC Family: Zone: KUAO Category: F Rank: P TW-2015 3,403 SqFt Length: Width: 40 Ft 50 Ft Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: TotalSamples:** 1 **Last Insp. Date:** 7/12/2018 Surveyed: 1 **Conditions: PCI:** 66 **Inspection Comments:** Sample Number: 01 R 3324.00 SqFt **PCI:** 66 Type: Area: **Sample Comments:**

48

48

57

L & T CR

L & T CR

WEATHERING

L

M

L

110.00 Ft

150.00 Ft

Network:	Aurora					Name:	Auro	ora State						
Branch:	TA3AU			Name:	Taxiwa	y A3 Aur	ora	Use:	TAXIW	AY	Area:		15,406 SqI	₹t
Section:	02	of	3	F	rom: T	TA3AU-0	1		To:	TAAU-02	2		Last Co	nst.: 9/3/200
Surface:	AC	Family:	OR-0 2015		Central-TW-	Zone:	KUAO		Cate	gory: F			Rank:	P
Area:	8	8,813 SqFt		Length:		183 Ft		Width:		40 Ft				
Slabs:		Slab Leng	gth:		Ft	Sl	lab Width:		Ft		Joint Le	ength:		Ft
Shoulder:		Street Ty	pe:			G	rade: 0				Lanes:	0		
Section Co	omments:													
	Date: 7/12/2			TotalSa	amples: 2			Surveye	ed: 2					
Condition Inspection	s: PCI: 8	80	e:				4403			PCI: 82				
Condition Inspection	s: PCI: 8 Comments:		e:	TotalSa		rea:	4403	Surveyo		PCI: 82				
Condition Inspection Sample No Sample Co	s: PCI: 8 Comments:	80	e: L	R		rea:	4403			PCI: 82				
Condition Inspection Sample No Sample Co	s: PCI: 8 Comments: umber: 01 omments:	80		R	Aı	rea: Ft	4403			PCI: 82				
Condition Inspection Sample No Sample Co 48 L &	s: PCI: 8 Comments: umber: 01 comments:	80	L L	R	190.00 4403.00	rea: Ft				PCI: 82				
Condition Inspection Sample No Sample Co 48 L & 57 WE Sample No	Comments: umber: 01 comments: the T CR EATHERING umber: 02	Турс	L L	R	190.00 4403.00	rea: Ft SqFt		.00 SqFt						
Condition Inspection Sample No Sample Co 48 L & 57 WE Sample No Sample Co	Comments: umber: 01 comments: the T CR EATHERING umber: 02	Турс	L L	R	190.00 4403.00	rea: Ft SqFt rea:		.00 SqFt						
Condition Inspection Sample No Sample Co 48 L & Sample No Sample Co 48 L & 48 L & 48 L &	s: PCI: 8 Comments: umber: 01 comments: & T CR EATHERING umber: 02 comments:	Турс	L L e:	R R	190.00 4403.00	rea: Ft SqFt rea: Ft Ft		.00 SqFt						

Network: Aurora Name: Aurora State **Branch:** TA3AU Name: Taxiway A3 Aurora Use: TAXIWAY Area: 15,406 SqFt **Section:** 03 of 3 From: TAAU-02 To: End **Last Const.:** 9/3/2007 OR-Cat2-AC-Central-TW- Zone: Surface: ACFamily: KUAO Category: F Rank: P 2015 3,190 SqFt Length: Width: 40 Ft Area: 51 Ft Slab Length: Ft Slab Width: Ft Slabs: Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 88 **Conditions: Inspection Comments:** Sample Number: 01 R 3190.00 SqFt PCI: 88 Type: Area: **Sample Comments:**

48 L & T CR L 60.00 Ft 57 WEATHERING L 3190.00 SqFt Network: Aurora Name: Aurora State **Branch:** TA4AU Name: Taxiway A4 Aurora Use: TAXIWAY Area: 12,352 SqFt 01 To: TA4AU-02 **Section:** of 2 From: **Runway 17/35 Last Const.:** 5/2/2005 OR-Cat2-AAC-Central-Surface: AAC Family: Zone: KUAO Category: F Rank: P TW-2015 Length: Width: 40 Ft 3,324 SqFt 50 Ft Area: Ft Slab Width: Slabs: Slab Length: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 58 **Inspection Comments:** Sample Number: 01 R 3324.00 SqFt PCI: 58 Type: Area: **Sample Comments:**

48 L & T CR L 150.00 Ft L & T CR 48 M 250.00 Ft 57 WEATHERING L 3324.00 SqFt

Network:	Aurora			Nan	e: Aurora Stat	е						
Branch:	TA4AU		Name:	Taxiway A4 A	urora I	Jse: TA	XIWAY	A	rea:	1	12,352 SqFt	
Section:	02	0:	f 2	From: TA4AU	-01	,	Го: ТАА	U-02			Last Const	: 9/3/2007
Surface:	AC	Family:	OR-Cat2-AC 2015	-Central-TW- Zon	e: KUAO	•	Category:	F			Rank: P	
Area:		9,028 SqFt	Length	: 183 F	t Widtl	:	40 F	t				
Slabs:		Slab Len	igth:	Ft	Slab Width:]	Ft		Joint Le	ngth:		Ft
Shoulder:		Street Ty	ype:		Grade: 0				Lanes:	0		
Section Cor	mments:											
Last Insp. I	Date: 7/12	/2018	Total	Samples: 2	Su	rveyed: 2						
Conditions	. DCI.	74		•		•						
		74		•		·						
	: PCI: Comments:			-		-						
Inspection (Comments:			Area:	4685.00 Sq		PCI:	83				
Inspection (Comments:					₹t		83				
Inspection (Sample Num Sample Com	Comments:		oe: R			₹t		83				
Inspection G Sample Nur Sample Cor	Comments: mber: 01 mments:	Туџ		Area:		₹t		83				
Inspection (Sample Num Sample Con 48 L & 57 WEA	Comments: mber: 01 mments: T CR ATHERING	Тур	oe: R L L	Area:								
Sample Nur Sample Cor 48 L & 57 WEA	Comments: mber: 01 mments: T CR ATHERING mber: 02	Туџ	oe: R L L	Area: 180.00 Ft 4685.00 SqFt	4685.00 Sq		PCI:					
Sample Nur Sample Cor 48 L & 57 WE Sample Nur Sample Cor	Comments: mber: 01 mments: T CR ATHERING mber: 02	Тур	oe: R L L	Area: 180.00 Ft 4685.00 SqFt	4685.00 Sq		PCI:					
Sample Nur Sample Cor 48 L & 57 WE Sample Nur Sample Cor 50 PAT	Comments: mber: 01 mments: T CR ATHERING mber: 02 mments:	Тур	De: R L L De: R	Area: 180.00 Ft 4685.00 SqFt Area:	4685.00 Sq		PCI:					
Sample Con 48 L & 57 WE Sample Num Sample Con 50 PAT 48 L &	Comments: mber: 01 mments: T CR ATHERING mber: 02 mments:	Тур	De: R L L De: R	Area: 180.00 Ft 4685.00 SqFt Area: 880.00 SqFt	4685.00 Sq		PCI:					

Network: Aurora Name: Aurora State **Branch:** TA5AU Name: Taxiway A5 Aurora Use: TAXIWAY 9,683 SqFt Area: **Section:** 01 of 3 From: TA5AU-02 To: Runway 35 End **Last Const.:** 5/2/2005 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: P 2015 2,520 SqFt Length: Width: 35 Ft 50 Ft Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 49 **Conditions: Inspection Comments:** Sample Number: 01 R 2520.00 SqFt **PCI**: 49 Type: Area: **Sample Comments:**

43

43

57

BLOCK CR

BLOCK CR

WEATHERING

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L

1260.00 SqFt

1260.00 SqFt

Network: Aurora Name: Aurora State **Branch:** TA5AU Name: Taxiway A5 Aurora Use: TAXIWAY 9,683 SqFt Area: To: TA5AU-03 **Section:** 02 of 3 From: TA5AU-01 **Last Const.:** 8/1/2008 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: P 2015 Length: Width: 35 Ft 3,188 SqFt 91 Ft Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 69 **Conditions: Inspection Comments:** Sample Number: 01 R 3188.00 SqFt **PCI**: 69 Type: Area: **Sample Comments:**

48

48

57

L & T CR

L & T CR

WEATHERING

L

M

L

110.00 Ft

110.00 Ft

Network:	Aurora				Name:	Auro	ora State				
Branch:	TA5AU		Name:	Taxiv	vay A5 Aurora		Use:	TAXIW	AY	Area:	9,683 SqFt
Section:	03	of	f 3	From:	Taxiway A			To:	TA5-02		Last Const.: 8/1/200
Surface:	AAC	Family:	OR-Cat2-AA TW-2015	C-Central-	Zone:	KUAO		Cate	gory: F		Rank: P
Area:		3,975 SqFt	Length	:	92 Ft		Width:		35 Ft		
Slabs:		Slab Len	gth:	Ft	Slab	Width:		Ft		Joint Length:	Ft
Shoulder:	:	Street Ty	ype:		Grad	le: 0				Lanes: 0	
Section Co	omments:										
Last Insp.	. Date: 7/12/	/2018	Total	Samples:	1		Surveye	d: 1			
Condition	s: PCI:	73									
	n Comments:										
Inspection					Area:	3975	5.00 SqFt		PCI: 73		
	umber: 01	Тур	oe: R		Aica.						
Sample N	umber: 01	Тур	e: R		AI ca.						
Sample No		Тур	oe: R L	90.00							
Sample No Sample Co	omments:	Тур			Ft						

Network: Aurora Name: Aurora State **Branch:** TAAAU Name: Taxiway AA Aurora Use: TAXIWAY Area: 7,284 SqFt To: TL03 **Section:** 01 of 1 From: TL01 **Last Const.:** 9/3/2016 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: P 2015 290 Ft Length: Width: 25 Ft 7,284 SqFt Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 2 Surveyed: 2 **Conditions: PCI:** 100 **Inspection Comments:** Sample Number: 01 R **PCI:** 100 Type: 3512.00 SqFt Area: **Sample Comments:** <No Distress>

3772.00 SqFt

PCI: 100

Sample Number: 02 **Sample Comments:**

Type:

R

Area:

Network: Aurora		Nam	e: Aurora State			
Branch: TAAU	Name:	Taxiway A Au		TAXIWAY A	Area: 17	/4,874 SqFt
Section: 01	of 3	From: TA1AU-	-02	To: T12AU-01		Last Const.: 9/3/2008
Surface: AC	Family: OR-Cat2-AC 2015	C-Central-TW- Zone	: KUAO	Category: F		Rank: P
Area: 56,78	S SqFt Length	1,626 Ft	Width:	35 Ft		
Slabs:	Slab Length:	Ft	Slab Width:	Ft	Joint Length:	Ft
Shoulder:	Street Type:		Grade: 0		Lanes: 0	
Section Comments:						
Last Insp. Date: 7/12/2018	3 Tota	ISamples: 11	Surveye	d: 4		
Conditions: PCI: 83						
Inspection Comments:						
Sample Number: 02	Type: R	Area:	5250.00 SqFt	PCI: 80		
Sample Comments:	• •					
48 L & T CR	L	260.00 Ft				
57 WEATHERING	L	5250.00 SqFt				
Sample Number: 04	Type: R	Area:	5250.00 SqFt	PCI: 87		
Sample Comments:						
48 L & T CR	L	120.00 Ft				
57 WEATHERING	L	5250.00 SqFt				
Sample Number: 06	Type: R	Area:	5250.00 SqFt	PCI: 90		
Sample Comments:						
48 L & T CR	L	40.00 Ft				
57 WEATHERING	L	5250.00 SqFt				
Sample Number: 09	Type: R	Area:	5250.00 SqFt	PCI: 76		
Sample Comments:						
48 L & T CR	L	250.00 Ft				

48

57

L & T CR

WEATHERING

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L

1.00 Ft

Network:	: Aurora			Name	e: Auro	ra State					
Branch:	TAAU		Name:	Taxiway A Au	rora	Use:	TAXIW	AY	Area:	174,874 SqFt	
Section:	02	of 3		From: TAAU-0)1		To:	TA4AU-0	2	Last Cons	t.: 9/3/2007
Surface:	AC		R-Cat2-AC-)15	-Central-TW- Zone	: KUAO		Cate	gory: F		Rank: P	
Area:	88,88	5 SqFt	Length:	2,540 Ft		Width:		35 Ft			
Slabs:		Slab Length	:	Ft	Slab Width:		Ft		Joint Le	ngth:	Ft
Shoulder	:	Street Type:			Grade: 0				Lanes:	0	
Section C	Comments:										
Last Insp	Date: 7/12/2018	3	Total	Samples: 17		Surveye	d: 5				
Condition	ns: PCI: 73										
Inspectio	on Comments:										
	Number: 03	Type:	R	Area:	5250	.00 SqFt		PCI: 71			
-	Comments:	1 јре.		111041.	0200	5411		101. /1			
_			_	4.50.00							
	& T CR & T CR		L M	150.00 Ft 150.00 Ft							
	EPRESSION		L	6.00 SqFt							
	EATHERING		L	5250.00 SqFt							
	Number: 07	Type:	R	Area:	5250	.00 SqFt		PCI: 74			
_	Comments:					-					
48 L	& T CR		L	110.00 Ft							
	& T CR		M	110.00 Ft							
57 W	/EATHERING		L	5250.00 SqFt							
Sample N	Number: 11	Type:	R	Area:	5250	.00 SqFt		PCI: 74			
Sample C	Comments:										
48 L	& T CR		L	450.00 Ft							
	/EATHERING		L	5250.00 SqFt							
Sample N	Number: 14	Туре:	R	Area:	5250	.00 SqFt		PCI: 70			
	Comments:										
48 L	& T CR		L	210.00 Ft							
	& T CR		M	160.00 Ft							
	/EATHERING		L	5250.00 SqFt							
Sample N	Number: 16	Type:	R	Area:	5250	.00 SqFt		PCI: 74			
-	Comments:					-					
57 W	/EATHERING		L	5250.00 SqFt							
	& T CR		L	100.00 Ft							
	& T CR		M	110.00 Ft							

Network:	Aurora			Nam	e: Aurora St	ate			
Branch:	TAAU		Name	: Taxiway A Au	rora	Use:	TAXIWAY	Area:	174,874 SqFt
Section:	03	of	3	From: TA4AU	-01		To: TAAU-04	1	Last Const.: 8/1/2008
Surface:	AC		OR-Cat2-A 2015	AC-Central-TW- Zone	e: KUAO		Category: F		Rank: P
Area:	2	9,204 SqFt	Leng	th: 834 F	t Wid	th:	35 Ft		
Slabs:		Slab Leng	gth:	Ft	Slab Width:		Ft	Joint Length	: Ft
Shoulder:		Street Typ	pe:		Grade: 0			Lanes: 0	
Section Co	mments:								
Last Insp. 1	Date: 7/12/2	2018	To	talSamples: 6	S	urveye	d: 3		
Conditions	: PCI:	69							
Inspection	Comments:								
Sample Nu	mber: 01	Туре	e: R	Area:	5250.00 S	qFt	PCI: 67		
Sample Co	mments:								
48 L &	T CR		L	250.00 Ft					
48 L &	T CR		M	220.00 Ft					
57 WE.	ATHERING		L	5250.00 SqFt					
Sample Nu	mber: 02	Туре	e: R	Area:	5250.00 S	qFt	PCI: 68		
Sample Co	mments:								
48 L&	T CR		L	280.00 Ft					
48 L &	T CR		M	200.00 Ft					
57 WE.	ATHERING		L	5250.00 SqFt					
Sample Nu	mber: 04	Туре	e: R	Area:	5250.00 S	qFt	PCI: 72		
Sample Co	mments:								
48 L &	T CR		L	150.00 Ft					
48 L &	T CR		M	140.00 Ft					
				5250.00 SqFt					

Network: Aurora Name: Aurora State **Branch:** TL1AU Name: Taxilane 01 Aurora Use: TAXIWAY Area: 9,921 SqFt 01 **Section:** of 1 From: TAA To: Hangars **Last Const.:** 9/3/2016 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 9,921 SqFt Length: Width: 25 Ft Area: 386 Ft Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 2 Surveyed: 2 **PCI:** 100 **Conditions: Inspection Comments:** Sample Number: 01 R 4648.00 SqFt **PCI:** 100 Type: Area: **Sample Comments:** <No Distress>

5273.00 SqFt

PCI: 100

Sample Number: 02 **Sample Comments:**

Type:

R

Area:

Network: Aurora Name: Aurora State **Branch:** TL2AU Name: Taxilane 02 Aurora Use: TAXIWAY Area: 10,673 SqFt 01 **Section:** of 1 From: TAA To: Hangars **Last Const.:** 9/3/2016 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 10,673 SqFt Length: 400 Ft Width: 25 Ft Area: Ft Ft Slabs: Slab Length: Slab Width: Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 2 Surveyed: 2 **PCI:** 100 **Conditions: Inspection Comments:** Sample Number: 01 R 4990.00 SqFt **PCI:** 100 Type: Area: **Sample Comments:** <No Distress>

5682.00 SqFt

PCI: 100

Sample Number: 02 **Sample Comments:**

Type:

R

Area:

Network: Aurora Name: Aurora State **Branch:** TL3AU Name: Taxilane 03 Aurora Use: TAXIWAY Area: 15,963 SqFt **Section:** 01 of 1 From: TAA To: Hangars **Last Const.:** 9/3/2016 Surface: ACFamily: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 15,963 SqFt Length: Width: 25 Ft Area: 546 Ft Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 3 Surveyed: 2 **Conditions: PCI:** 100 **Inspection Comments:** 5823.00 SqFt Sample Number: 02 R **PCI:** 100 Type: Area: **Sample Comments:** <No Distress>

5561.00 SqFt

PCI: 100

Sample Number: 03
Sample Comments:

Type:

R

Area:

Network: Aurora Name: Aurora State **Branch:** TNWYLEEAU Name: North Wylee Taxiway Aurora Use: TAXIWAY Area: 3,465 SqFt **Section:** of 1 From: TAAU-01 To: Hangars **Last Const.:** 9/3/2008 Surface: AC Family: OR-Cat2-AC-Central-TW- Zone: KUAO Category: F Rank: S 2015 3,465 SqFt Length: Width: 26 Ft 66 Ft Area: Ft Slabs: Slab Length: Slab Width: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 75 **Inspection Comments:** Sample Number: 01 R 3465.00 SqFt PCI: 75 Type: Area: **Sample Comments:** 48 L & T CR L 50.00 Ft

L & T CR

WEATHERING

M

L

60.00 Ft

3465.00 SqFt

48

57

Network: Aurora Name: Aurora State **Branch:** TSWYLEEAU Name: South Wylee Taxiway Aurora Use: TAXIWAY Area: 3,237 SqFt TAAU-01 **Section:** of 1 From: To: Hangars **Last Const.:** 9/3/2008 OR-Cat2-AC-Central-TW- Zone: Surface: AC Family: KUAO Category: F Rank: S 2015 66 Ft Length: Width: 25 Ft Area: 3,237 SqFt Ft Slab Width: Slabs: Slab Length: Ft Joint Length: Ft Shoulder: **Street Type:** Grade: 0 Lanes: 0 **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **PCI:** 94 **Conditions: Inspection Comments:** Sample Number: 01 R 3237.00 SqFt **PCI:** 94 Type: Area:

Sample Comments:

57 WEATHERING L 3237.00 SqFt

Network: Aurora Name: Aurora State **Branch:** TWILLAVAU Name: Willamette Aviation Taxiway Use: TAXIWAY3,777 SqFt Area: Aurora TAAU-01 To: Hangars Section: 01 of 1 From: **Last Const.:** 9/3/2008 AC Family: OR-Cat2-AC-Central-TW- Zone: Category: F Rank: P Surface: KUAO 2015 Area: 3,777 SqFt Length: 70 Ft Width: 42 Ft Slab Width: Slabs: Slab Length: Ft Ft Joint Length: Ft Grade: Shoulder: **Street Type:** 0 Lanes: 0 **Section Comments: Last Insp. Date:** 7/12/2018 **TotalSamples:** 1 Surveyed: 1 **Conditions: PCI:** 89 **Inspection Comments:** Sample Number: 01 Type: R 3777.00 SqFt **PCI:** 89 Area:

Sample Comments:

 48
 L & T CR
 L
 30.00 Ft

 57
 WEATHERING
 L
 3777.00 SqFt

September 16, 2019

6289 AURORA STATE AIRPORT RUNWAY 17-35 PCN EVALUATION
(ISSUED 11/12/2019)

Century West Engineering Corporation 5331 SW Macadam Avenue, Suite 287 Portland, OR 97239

Attention: James Kirby, PE

Senior Project Manager

SUBJECT: Pavement Classification Number (PCN) Evaluation of Runway 17-35

Aurora State Airport (UAO)

Aurora, Oregon

As requested, GRI conducted a pavement evaluation at Aurora State Airport (UAO) in support of the Oregon Department of Aviation (ODA) to develop a pavement classification number (PCN) for Runway 17-35.

PROJECT DESCRIPTION

Our work included review of relevant ODA records for Runway 17-35, falling weight deflectometer (FWD) testing, core explorations, and engineering analyses in accordance with Federal Aviation Administration (FAA) Advisory Circular 150/5335-5C, *Standardized Method of Reporting Airport Pavement Strength – PCN*. According to the FAA, the PCN is a number that expresses the load-carrying capacity of a pavement for unrestricted operations. We determined the PCN using the Technical Evaluation Method specified in Advisory Circular 150/5335-5C.

BACKGROUND

Based on information provided in the ODA pavement evaluation/maintenance management program report prepared by Pavement Consultant Inc. in 2018, a 4,100-ft-long segment on the north end of the runway was first constructed in 1943 and in 1993, a 900-ft-long extension was built to the south. The last major rehabilitation on the runway was conducted in 2005 and generally consisted of a 2- to 3-in. overlay.

The current Airport Master Record, FAA Form 5010, lists the gross weight limit for a single-wheel, main-gear aircraft and a dual-wheel, main-gear aircraft at 30,000 and 45,000 lbs, respectively. UAO currently does not have an established PCN.

FIELD WORK

Site Reconnaissance

A visual pavement reconnaissance was performed by GRI engineers on August 12, 2019, to assess the general surface condition of the pavements within the project and to identify core exploration locations.

Falling Weight Deflectometer Tests

GRI conducted FWD testing on August 20, 2019, along the full length of the runway. The testing was conducted in accordance with FAA Advisory Circular 150/5370-11b, *Use of Nondestructive Testing in the Evaluation of Airport Pavements*, using our KUAB 2m Model 150 FWD device.

FWD testing was completed along test lines located at 7 ft west and 12 ft east of the runway centerline. The tests were spaced at approximately 200-ft intervals within the runway keel section. The approximate locations of the test lines are shown on Figure 1.

The FWD test procedures are described in Appendix A. The data were normalized to a 30,000-lb load basis and the FWD deflection data are shown in Table 1A.

We also reviewed the load-response data measured by the FWD to provide a preliminary understanding of the overall stiffness of the pavement structure. Although this information does not provide information about the stiffness of individual soil and pavement layers, it does provide a quick assessment of the overall stiffness of the pavement system to gauge the variability of pavement stiffness within a particular pavement facility. Impact stiffness modulus (ISM) is inversely proportional to deflection and is therefore a direct measurement of the combined stiffness, or resistance to deflection induced by FWD loading, of the pavement and subgrade soils. As such, it is usually a relative measure of the pavement's ability to support loads, i.e., high ISM modulus values usually correspond to high pavement strength and vice versa. The profile of relative pavement strength along the two FWD test lines, as measured by resistance to deflection under FWD loading, is plotted for each FWD test location on Figure 4A. Additional discussion regarding ISM is provided in Appendix A.

Coring Explorations

General. On August 20, 2019, GRI conducted three core explorations, all of which were located over cracks. The approximate locations of the explorations are shown on the Site Plan, Figure 1. Details of our field investigations are further discussed in Appendix A of this report and the core explorations are summarized in Table 1.

Core No.	FWD Test	Test Line	Station	Asphalt Concrete Thickness, in.	Aggregate Base Thickness, in.	Drilled Over a Crack?	Depth of Crack, in.
B-1	26	7 ft west	56 + 81	8.75	15.00	Yes	2.50
B-2	16	7 ft west	39 + 51	9.00	15.00	Yes	3.25
B-3	32	12 ft east	19 + 41	9.00	15.00	Yes	2.50

Table 1: SUMMARY OF CORING EXPLORATION RESULTS

Existing Pavement Conditions

Overall, the pavement surface of Runway 17-35 appears to be in good condition. The primary distresses observed on the runway are low- to medium-severity longitudinal cracking, primarily at paving-panel joints or along the centerline; low-severity weathering; and isolated low-severity alligator cracking within the gear paths.



Since the alligator cracking within the gear paths (noted above) is a load-associated distress, in our opinion, it warranted further investigation and we therefore conducted the three core explorations in areas of alligator cracking on the runway. As shown in Table 1 and the photo logs on Figures 1A through 3A in Appendix A, the cracking is top down and extends to a depth of 2.5 in. in cores B-1 and B-3 and to a depth of 3.25 in. in B-2. These types of cracks may be induced by excessive shear stresses imposed by aircraft wheel loads at the runway surface and can typically be repaired by milling to the depth of cracking and overlaying. In our opinion, pavement exhibiting this type of distress should be rehabilitated when the cracking progresses to the point that spalling begins to occur and therefore represents a significant Foreign Object Damage (FOD) potential. The core samples also exhibit delamination (separation of asphalt concrete [AC] layers) at a depth of 2.5 and 3.25 in. in cores B-2 and B-3, respectively. The depth of delamination generally agrees with the thickness of the 2005 overlay.

DESIGN PROCEDURES AND ANALYSIS

Traffic Loading

Century West Engineering Corporation (CWE) provided an estimate of the aircraft traffic-volume data consisting of the number of operations (i.e., either an arrival or departure) for Runway 17-35 in 2018 from the FAA Traffic Flow Management System Counts (TFMSC). Our traffic-loading estimate is based on an annual growth rate of 1.58% per year, which is based on the aviation forecasts provided in the current master plan for UAO (WHPacific, 2012).

The COMFAA 3.0 software used to compute the PCN has inputs for each aircraft type (in the mix), which include the type of aircraft, gross weight, and number of annual departures over a 20-year period. The program does not take into account the annual growth rate, so we calculated the total departures from 2020 to 2040 to determine the equivalent annual number of departures for the analysis. The aircraft mix and annual number of departures we input into COMFAA are provided in Table 2.

Table 2: RUNWAY 17-35: AIRCRAFT TYPES AND DEPARTURE VOLUMES

	Maximum		2018		Values Entered int	o COMFAA
Aircraft Type	Takeoff Weight, lbs	Design Aircraft for COMFAA	Annual Operations	2040 Annual Operations	Equivalent Airplane	Annual # of Departures
Bombardier Global Express	92,500	Gulfstream G-V	50	61	Gulfstream G-V	64
Gulfstream G600	91,600	Gulfstream G-V	2	3		
Gulfstream V	76,850	Gulfstream G-IV	2	3	Gulfstream G-IV	7
Gulfstream IV	73,200	Gulfstream G-IV	2	3	Gunstream G-IV	/
Dassault Falcon 900	45,503	Falcon-900	68	83	Falcon-900	83
Bombardier Challenger 600	45,100	Challenger CL- 604	58	70	Challanger CL 604	176
Bombardier Challenger 300	38,850	Challenger CL- 604	88	106	Challenger CL-604	176
Dassault Falcon 2000	41,000	Falcon-2000	34	42	Falcon-2000	42
Dassault Falcon 50	37,480	Falcon-50	276	332	Falcon-50	424
Dassault Falcon 20	28,650	Falcon-50	76	92	i aicon-30	424
Cessna Citation 750	36,600	Citation X	104	126	Citation X	292



	Maximum		2018		Values Entered in	to COMFAA
Aircraft Type	Takeoff Weight, lbs	Design Aircraft for COMFAA	Annual Operations	2040 Annual Operations	Equivalent Airplane	Annual # of Departures
Cessna Citation 680	30 <i>,77</i> 5	Citation X	138	167		
Hawker 800	28,000	Hawker-800	34	42	Hawker-800	42
Gulfstream G150	26,100	D-35	80	97	D-35	97
Astra 1125	24,650	D-30	96	11 <i>7</i>	D-30	11 <i>7</i>
Cessna Citation 650	22,000	Citation VI/VII	98	119	Citation VI/VII	119
Learjet 60	23,500	Learjet-55	30	36		
Learjet 55	21,500	Learjet-55	4	6	Learjet-55	57
Learjet 75	21,500	Learjet-55	12	15		
Learjet 45	20,500	Learjet-35A/65A	110	133		
Learjet 35	18,000	Learjet-35A/65A	8	10	Learjet-35A/65A	254
Learjet 31	15,500	Learjet-35A/65A	92	111		
Cessna Citation 560	20,000	Citation 550B	704	847	C't t' FEOD	1.100
Cessna Citation 550	13,300	Citation 550B	212	255	Citation 550B	1,102
Phenom 300/ Embraer 300	17,968	D-25	56	68	D-25	68
		Total Operations:	2,434			2,944

Backcalculation Analysis of FWD Test Data

The elastic moduli of the subgrade soil at the boring locations were backcalculated from the FWD test data. The average minus-one standard deviation subgrade moduli for each analysis unit (design modulus) are shown at the bottom of the backcalculation analysis results in Table 2A in Appendix A.

PAVEMENT CLASSIFICATION NUMBER (PCN) CALCULATIONS

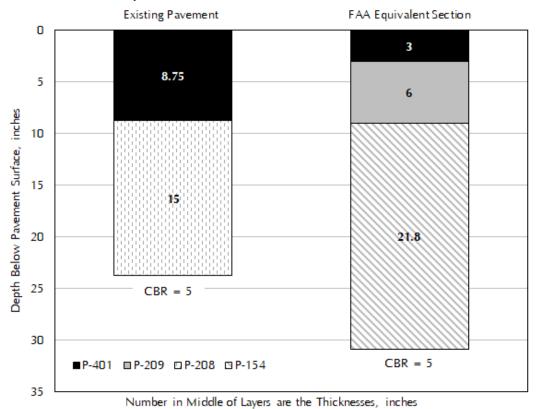
As requested by the ODA, we calculated the PCN for Runway 17-35 for each aircraft in the fleet mix based on the critical pavement-layer thickness and subgrade-support characteristics developed herein. The California bearing ratio (CBR) used in the PCN analysis is based on the backcalculated design modulus from Analysis Unit 2 in Table 2A in Appendix A and was calculated using the typical correlation between CBR and Resilient Modulus (Mr) and the correlation adopted by the FAA in Advisory Circular 150/5320-6F, Airport Pavement Design and Evaluation, which is represented by the following:

$$CBR = M_r / 1,500$$

The analysis was conducted using the FAA's Support Spreadsheet, COMFAA 3.0. The pavement-layer thicknesses were converted into an equivalent pavement section using the appropriate subgrade-support code and the default values for the conversion factors given in Advisory Circular 150/5335-5C. Based on our analysis, the equivalent pavement section is also shown on the following figure.



EQUIVALENT PAVEMENT SECTION FOR RUNWAY 17-35



Results of the PCN computations summarized in Table 3 are based on the departure traffic provided by CWE. For Runway 17-35, we recommend publishing the PCN value shown in Table 3. The corresponding PCN elements of the runway are summarized in Form 5010 (Table 1B) in Appendix B.

Table 3: RECOMMENDED UPDATES TO FAA FORM 5010 FOR UAO RUNWAY 17-35

		Aircraft Gross Weight, thousands lbs			
Runway	PCN	Single Wheel Main Gear	Dual Wheel Main Gear		
17-35	40/F/C/X/T	102	145		

Our recommended single-wheel, main-gear and dual-wheel, main-gear aircraft gross weights are 102,000 and 143,000 lbs, respectively. The increase in wheel-load capacity (as compared to the current Airport Master Record, FAA Form 5010) is likely due to the increased structural capacity related to the 2005 overlay. Additional discussion regarding the PCN methodology and reporting is provided in Appendix B.

LIMITATIONS

This pavement report has been prepared for use by the Oregon Department of Aviation and Century West Engineering Corporation and should not be relied upon by any other entity without the written permission of an authorized representative. The scope is limited to the specific project and location described herein, and our description of the project represents our understanding of the significant aspects of the project relevant to the analysis of the pavements at the time of publication.



PCN system is only intended as a method that airport operators can use to evaluate acceptable operations of aircraft. It is not intended as a pavement design or pavement evaluation procedure, nor does it restrict or replace the methodology used to design or evaluate a pavement structure.

Our work has been performed in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing under similar conditions in the locale. The results and conclusions submitted in this report are based on the data obtained from our sources of information discussed in this report. No other warranty, expressed or implied, is made.

Please contact the undersigned if you have any questions regarding this report or any other pavement considerations associated with this project.

Submitted for GRI,



Renews 12/2020

Michael J. Maloney, PE Principal Or was Married as

Lindsi A. Hammond, PE Associate

This document has been submitted electronically.

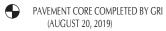
References

WHPacific, Inc., 2012, Aurora State Airport, Airport Master Plan Update.

Pavement Consultants Inc., 2018, 2018 Pavement Evaluation / Maintenance Management Program: Aurora State Airport.

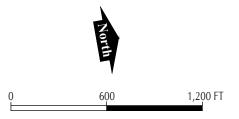






FWD TESTING COMPLETED BY GRI (AUGUST 20, 2019)

SITE PLAN FROM GOOGLE EARTH (IMAGE DATE JULY 2018)





SITE PLAN

SEP. 2019 JOB NO. 6289 FIG. 1



APPENDIX A

FIELD EXPLORATIONS AND FWD DATA

FIELD EXPLORATIONS

Existing pavement and subsurface conditions on Runway 17-35 were investigated by GRI on August 20, 2019, with three core explorations, designated B-1 through B-3. The approximate locations of the explorations are shown on the Site Plan, Figure 1. The field exploration and laboratory programs completed for this project are described below.

Pavement Core Explorations

The pavement was cored at each exploration location to assist in evaluation of the type of cracking and/or the thickness and condition of the asphalt concrete (AC). The pavement was cored using an electric drill owned and operated by GRI. Photographs of the core locations and core samples are shown on Figures 1A through 3A. Below the AC, we excavated to a maximum total depth of 24 in. below ground surface to observe the condition of the aggregate base (AB) and subgrade, if encountered. The subgrade was not encountered during our explorations and the AB was classified as silty sandy gravel ranging from angular to rounded and up to 1 to 1.5 in. in diameter.

FWD DATA

Falling weight deflectometer (FWD) tests were conducted by GRI on August 20, 2019, using our KUAB Model 150 FWD. The annual reference calibration for the FWD was accomplished in October 2019 at the KUAB manufacturing facility in Savoy, Illinois.

The FWD testing on Runway 17-35 was accomplished along test lines located at 7 ft west and 12 ft east of the runway centerline. The tests were completed at approximately 200-ft intervals within the keel section of the runway.

General

Geodetic coordinates of all test locations were measured from GPS signal using a submeter-capable Trimble™ GPS receiver with the antenna mounted on the FWD above the load plate.

The FWD load is generated by a two-mass/two-buffer, falling-weight system that produces a nearly haversine-shaped load-pulse waveform. The buffer and weight combination used for these tests produces a load rise time of approximately 14 milliseconds with an equivalent haversine frequency of approximately 32 Hz. The load pulse was applied to the pavement surface through a 450-mm-diameter (8.86-in.-radius), four-part, segmented plate designed to apply uniform surface pressure distribution despite irregularities in the pavement surface. Air temperature and pavement surface temperature (the latter measured by infrared thermometer) were recorded for each test.

Test Data

The average deflections from the two nominal 32,000-lb impact loads were linearly normalized to a 30-kip (30,000-lb) load basis and are tabulated in Table 1A of this appendix. The measurement units for the test



data are distance in feet, deflections in mil units (1 mil = 0.001 in.), load in pounds, sensor distance in inches, load plate radius in inches, and temperature in degrees Fahrenheit.

Impact Stiffness Modulus (ISM)

The Impact Stiffness Modulus (ISM) shown in units of kips per square inch (ksi) is the composite stiffness, or dynamic plate bearing modulus, of all the materials beneath the pavement/roadway surface. It is computed using the Boussinesq formula for surface deflection beneath the center of a uniformly loaded circular area on a linear-elastic half space, with a Poisson's ratio of 0.50. The surface deflection measured at the center of the FWD load plate (D0) was used to compute the surface modulus. The magnitude of the ISM is inversely proportional to deflection and comparable to the elastic modulus. The difference between the pavement ISM and elastic modulus is that the elastic modulus represents the elastic load-deformation response of an individual pavement layer or the subgrade soil, whereas the pavement ISM represents the composite elastic load-deformation response of all materials (pavement layers and subgrade soil) below the pavement surface. Therefore, the ISM (as computed from the deflection measured beneath the FWD load plate) cannot be taken as representative of the elastic modulus of any single pavement layer or the subgrade soil. However, since it is a measurement of the combined stiffness of the pavement structure and subgrade soil, it is often useful for evaluation of variation in pavement stiffness and for assessment of relative pavement strength. Plots of the ISMs are shown on Figure 4A.



Table 1A - FWD NORMALIZED DEFLECTION TEST DATA RUNWAY 17-35: AURORA STATE AIRPORT (UAO)

Test Section: RW 17-35

Start Point: North edge of runway, 10+00

Test Date: 8/20/2019

Test File: 6289-Aurora Airport.fwd

Load Plate Radius, in: 8.86

Sensor Distance, in: 0 12 18 24 36 48 60 72

Deflections Normalized to 30000 lbf Basis

		1		1		CHECTIONS	TTOTTIATIZ	<u>eu 10 300</u>	l Dus		1	C (C (
	.											Surface		Surface	161.4	
	Test		6	D 4 1	D 0 '1	D 2 '1	D 4 11	D = 1	D 6 11	D =	D 0 '1	Temp.,	T ·	Modulus	ISM,	
Test No.	Station	Test Line	Core	D 1, mils		,						°F	Time	, Ksi		Comments
1	10+50	7' w		28.54	24.85	21.17	18.56	13.73	10.05	7.37	5.54	68	1:24:59	57	1,051	7' west
2	12 + 50	7' w		25.28	20.28	16.82	14.62	10.56	7.81	5.80	4.50	71	1:26:36		1,187	
3	14+49	7' w		30.42	25.52	21.55	18.73	13.50	9.84	7.24	5.55	71	1:27:52	53	986	
4	16+51	7' w		29.35	24.82	20.94	18.25	13.29	9.74	7.15	5.47	71	1:29:09	55	1,022	
5	18 + 50	7' w		24.65	20.46	17.12	14.81	10.62	7.71	5.71	4.47	71	1:30:14	66	1,217	
6	20 + 56	7' w		27.93	22.60	18.54	15.81	11.05	7.98	5.87	4.66	71	1:31:20	58	1,074	
7	22 + 50	7' w		25.72	21.22	17.71	15.34	11.10	8.13	6.06	4.70	71	1:32:26	63	1,166	
8	24 + 51	7' w		26.54	21.58	17.98	15.18	10.67	7.71	5.71	4.47	71	1:33:33	61	1,130	
9	26+53	7' w		26.28	20.74	17.15	14.64	10.47	7.67	5.83	4.64	70	1:34:39	62	1,142	
10	28 + 55	7' w		26.82	22.10	18.49	15.98	11.58	8.49	6.34	4.95	71	1:35:42	60	1,119	
11	30 + 54	7' w		26.27	21.60	18.22	15.84	11.70	8.66	6.45	4.96	71	1:37:01	62	1,142	
12	32 + 54	7' w		30.95	25.88	21.81	19.07	13.97	10.26	7.67	5.78	71	1:38:07	52	969	
13	34 + 52	7' w		36.96	27.64	22.18	18.81	13.26	9.67	7.12	5.56	71	1:39:22	44	812	
14	36 + 57	7' w		32.41	26.67	22.42	19.26	13.87	10.02	7.26	5.44	70	1:40:28	50	926	
15	38 + 52	7' w		28.76	23.55	19.60	16.84	12.06	8.67	6.34	4.88	70	1:41:38	56	1,043	
16	39 + 51	7' w	B-2	34.09	27.13	22.55	19.48	14.13	10.46	7.65	5.72	70	1:43:21	47		B-2
17	40 + 51	7' w		27.27	22.43	18.67	16.13	11.60	8.44	6.11	4.75	70	1:44:29	59	1,100	
18	42 + 51	7' w		31.58	25.74	21.56	18.44	13.11	9.35	6.80	5.10	70	1:45:38	51	950	
19	44 + 51	7' w		29.21	23.02	18.77	15.98	11.24	7.90	5.76	4.52	70	1:46:46	55	1,027	
20	46 + 50	7' w		29.41	23.54	19.35	16.44	11.40	7.92	5.78	4.50	70	1:47:53	55	1,020	
21	48 + 52	7' w		28.25	23.01	19.08	16.26	11.38	8.17	6.06	4.66	70	1:49:02	57	1,062	
22	50 + 52	7' w		39.77	29.04	22.94	19.04	12.53	8.69	6.21	4.86	70	1:50:10	41	754	
23	52 + 50	7' w		34.37	27.28	22.48	18.86	12.83	8.94	6.47	5.08	70	1:51:20	47	873	
24	54 + 51	7' w		44.23	34.59	27.53	22.75	14.74	9.70	6.77	5.20	69	1:52:33	37	678	
25	56 + 40	7' w		37.32	28.83	22.75	18.62	11.88	7.81	5.61	4.42	67	1:53:49	43	804	
26	56 + 81	7' w	B-1	35.88	28.79	23.20	19.31	12.57	8.38	5.79	4.55	70	1:55:03	45	836	B-1
27	58 + 50	7' w		35.45	27.78	22.05	18.05	11.74	7.82	5.60	4.34	65	1:56:22	46	846	5875 = s end end 7' west
28	11 + 50	12' e		25.22	21.35	18.22	15.93	11.88	8.90	6.66	5.09	68	2:05:27	64	1,190	12' east
29	13 + 50	12' e		30.01	25.29	21.29	18.67	13.66	10.11	7.43	5.70	70	2:07:03	54	1,000	
30	15 + 51	12' e		30.03	25.22	21.26	18.42	13.46	9.89	7.28	5.64	70	2:08:15	54	999	
31	17 + 53	12' e		28.42	22.94	19.00	16.27	11.53	8.38	6.20	4.83	70	2:09:28	5 <i>7</i>	1,056	
32	19 + 41	12' e	B-3	34.02	25.85	20.87	17.26	11.79	8.33	6.13	4.74	70	2:13:56	48	882	B-3
33	21 + 50	12' e		21.06	17.31	14.42	12.49	9.07	6.79	5.19	4.17	70	2:16:05	77	1,425	
34	23 + 52	12' e		25.55	21.01	17.53	15.14	11.13	8.27	6.23	4.95	70	2:17:18	63	1,174	
35	25 + 52	12' e		21.98	17.91	15.02	13.04	9.69	7.31	5.60	4.43	69	2:18:26	74	1,365	
36	27 + 51	12' e		26.27	20.79	16.87	14.33	10.21	7.48	5.62	4.44	69	2:19:33	62	1,142	
37	29 + 50	12' e		34.66	28.16	23.24	19.76	13.95	10.10	7.48	5.79	69	2:20:42	47	866	



Table 1A - FWD NORMALIZED DEFLECTION TEST DATA RUNWAY 17-35: AURORA STATE AIRPORT (UAO)

Deflections Normalized to 30000 lbf Basis

												Surface		Surface		
	Test											Temp.,		Modulus	ISM,	
Test No.	Station	Test Line	Core	D 1, mils	D 2, mils	D 3, mils	D 4, mils	D 5, mils	D 6, mils	D 7, mils	D 8, mils	°F	Time	, Ksi	kips/in	Comments
38	31 + 52	12' e		27.24	22.35	18.84	16.39	12.19	9.20	6.99	5.47	69	2:21:52	59	1,101	
39	33 + 49	12' e		26.34	21.87	18.38	15.90	11.64	8.78	6.71	5.25	69	2:23:00	61	1,139	
40	35 + 53	12' e		24.64	20.22	16.91	14.67	10.73	8.01	6.08	4.83	69	2:24:09	66	1,218	
41	37 + 51	12' e		29.65	24.86	20.96	18.32	13.45	9.99	7.38	5.60	69	2:25:16	55	1,012	
42	39 + 50	12' e		25.27	21.38	17.99	15.86	11.68	8.77	6.56	5.13	69	2:26:26	64	1,187	
43	41 + 51	12' e		25.80	21.67	18.35	15.90	11.67	8.62	6.43	4.94	69	2:27:34	63	1,163	
44	43 + 50	12' e		27.58	23.19	19.57	17.18	12.51	9.22	6.76	5.14	69	2:28:38	59	1,088	
45	45 + 51	12' e		26.22	21.41	17.71	15.13	10.72	7.77	5.72	4.51	69	2:29:48	62	1,144	
46	47 + 54	12' e		28.02	22.49	18.48	15.60	10.83	7.75	5.68	4.46	69	2:30:56	58	1,071	
47	49 + 51	12' e		27.34	22.44	18.36	15.67	11.04	7.94	5.90	4.62	69	2:32:04	59	1,097	
48	51 + 53	12' e		30.35	24.69	20.12	17.00	11.60	8.11	5.96	4.66	69	2:33:11	53	988	
49	53 + 55	12' e		31.95	26.02	21.17	17.69	11.99	8.46	6.17	4.85	69	2:34:18	51	939	
50	55 + 50	12' e		36.26	28.03	22.28	18.48	12.16	8.34	6.04	4.75	69	2:35:31	45	827	
51	57 + 51	12' e		32.67	26.40	21.38	17.62	11.50	7.75	5.50	4.31	67	2:36:47	49	918	5878 = s end end 12' east



Table 2A - BACKCALCULATION ANALYSIS SUMMARY RUNWAY 17-35: AURORA STATE AIRPORT (UAO)

Runway 17-35: Aurora State Airport (UAO)

Based on FWD Testing Conducted: 8/20/2019 Start Station: North edge of runway, 10+00

	l	I		l I		1	Ī	T
FWD Test #	Test Station	Test Line	Core Exploration	Analysis Unit	D0, mils	AC Thickness, inches	AB Thickness, inches	Subgrade Modulus, psi
1	10 + 50	7' w		1	28.54	9.00	15.00	10,402
2	12 + 50	7' w		1	25.28	9.00	15.00	15,441
3	14+49	7' w		1	30.42	9.00	15.00	11,553
4	16+51	7' w		1	29.35	9.00	15.00	11,570
5	18 + 50	7' w		1	24.65	9.00	15.00	12,902
6	20 + 56	7' w		1	27.93	9.00	15.00	11 <i>,7</i> 68
7	22 + 50	7' w		1	25.72	9.00	15.00	14,630
8	24+51	7' w		1	26.54	9.00	15.00	12,567
9	26 + 53	7' w		1	26.28	9.00	15.00	15,004
10	28 + 55	7' w		1	26.82	9.00	15.00	14,486
11	30 + 54	7' w		1	26.27	9.00	15.00	13,228
12	32 + 54	7' w		1	30.95	9.00	15.00	10,155
13	34 + 52	7' w		1	36.96	9.00	15.00	9,847
14	36 + 57	7' w		1	32.41	9.00	15.00	10,365
15	38 + 52	7' w		1	28.76	9.00	15.00	10,556
16	39 + 51	7' w	B-2	1	34.09	9.00	15.00	9,726
17	40 + 51	7' w		1	27.27	9.00	15.00	10,489
18	42 + 51	7' w		1	31.58	9.00	15.00	11,108
19	44 + 51	7' w		1	29.21	9.00	15.00	11,314
20	46 + 50	7' w		1	29.41	9.00	15.00	11,087
21	48 + 52	7' w		1	28.25	9.00	15.00	14,129
22	50 + 52	7' w		2	39.77	8.75	15.00	8,814
23	52 + 50	7' w		2	34.37	8.75	15.00	9,367
24	54 + 51	7' w		2	44.23	8.75	15.00	6,713
25	56+40	7' w		2	37.32	8.75	15.00	9,796
26	56+81	7' w	B-1	2	35.88	8.75	15.00	<i>7,</i> 615
27	58 + 50	7' w		2	35.45	8.75	15.00	9,512
28	11 + 50	12' e		1	25.22	9.00	15.00	12,541
29	13 + 50	12' e		1	30.01	9.00	15.00	11,399
30	15+51	12' e		1	30.03	9.00	15.00	9,781
31	17+53	12' e		1	28.42	9.00	15.00	11,645
32	19+41	12' e	B-3	1	34.02	9.00	15.00	10,977
33	21 + 50	12' e		1	21.06	9.00	15.00	17,720
34	23 + 52	12' e		1	25.55	9.00	15.00	13,364
35	25 + 52	12' e		1	21.98	9.00	15.00	14,811
36	27 + 51	12' e		1	26.27	9.00	15.00	14,236
37	29 + 50	12' e		1	34.66	9.00	15.00	11,837
38	31 + 52	12' e		1	27.24	9.00	15.00	10,942
39	33+49	12' e		1	26.34	9.00	15.00	11,421
40	35 + 53	12' e		1	24.64	9.00	15.00	14,477
41	37+51	12' e		1	29.65	9.00	15.00	10,835
42	39 + 50	12' e		1	25.27	9.00	15.00	11,501
43	41 + 51	12' e		1	25.80	9.00	15.00	13,236
44	43 + 50	12' e		1	27.58	9.00	15.00	11,913



Table 2A - BACKCALCULATION ANALYSIS SUMMARY RUNWAY 17-35: AURORA STATE AIRPORT (UAO)

FWD Test #	Test Station	Test Line	Core Exploration	Analysis Unit	D0, mils	AC Thickness, inches	AB Thickness, inches	Subgrade Modulus, psi
45	45 + 51	12' e		1	26.22	9.00	15.00	12,250
46	47 + 54	12' e		1	28.02	9.00	15.00	11,825
47	49 + 51	12' e		1	27.34	9.00	15.00	12,606
48	51 + 53	12' e		2	30.35	8.75	15.00	11,238
49	53 + 55	12' e		2	31.95	8.75	15.00	10,326
50	55 + 50	12' e		2	36.26	8.75	15.00	9,761
51	57 + 51	12' e		2	32.67	8.75	15.00	9,341

Statistical Summary

Structura			PAVER PMP	Average D0,	Average AC	Average AB	Average Subgrade
	From Sta	To Sta	Unit	mils	Thickness, in.	Thickness, in.	Modulus, psi
1	0 + 00	49 + 51	R17AU-01	28.10	9.00	15.00	12,235
2	0 + 00	58 + 50	R17AU-02	35.83	8.75	15.00	9,248

Design Subgrade Resilient Modulus

Structura I Unit #	From	То	PAVER PMP Unit	Average Subgrade Modulus, psi	Standard Deviation, psi	Average Subgrade — Standard Deviation, psi	CBR, Mr (psi)/1500
1	10 + 50	49 + 51	R17AU-01	12,235	1,800	10,435	7
2	50 + 52	58 + 50	R17AU-02	9,248	1,294	7,955	5





Core B-1 (RW 17-35 8' West of Centerline, Station 56+81, FWD 26)



B-1 (Pavement Core Sample, 8.75 in.)



PAVEMENT CORE PHOTOGRAPHS

SEP. 2019 JOB NO. 6289 FIG. 1A



Core B-2 (RW 17-35 8' West of Centerline, Station 39+51, FWD 16)



B-2 (Pavement Core Sample, 9.0 in.)



PAVEMENT CORE PHOTOGRAPHS

SEP. 2019 JOB NO. 6289 FIG. 2A



Core B-3 (RW 17-35 12' East of Centerline, Station 19+41, FWD 32)

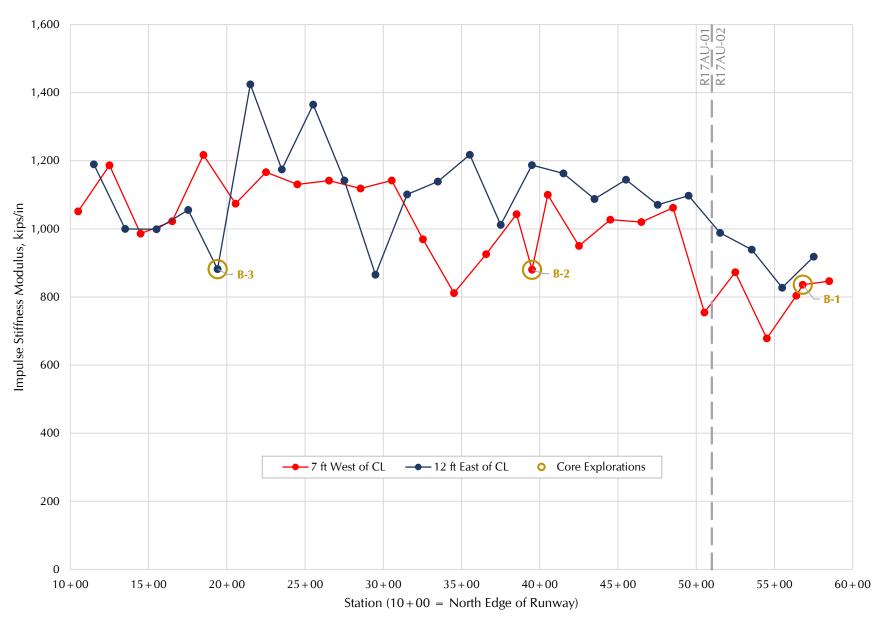


B-3 (Pavement Core Sample, 9.0 in.)



PAVEMENT CORE PHOTOGRAPHS

SEP. 2019 JOB NO. 6289 FIG. 3A





IMPULSE STIFFNESS MODULUS

SEP. 2019 JOB NO. 6289 FIG. 4A



APPENDIX B

PAVEMENT CLASSIFICATION NUMBER ANALYSIS

BACKGROUND

In 2014, the FAA instituted a requirement that Part 139-certified airports be assigned pavement classification number (PCN) data. The PCN is required because the United States is a member state of the International Civil Aviation Organization (ICAO), the international regulatory body for air traffic. ICAO adopted the Aircraft Classification Number (ACN)-Pavement Classification Number (ACN-PCN) method to allow any airport a standardized method for reporting the effect of aircraft that use the facility, as well as the load-carrying capacity of the pavement (ICAO, 1999).

The ACN is a number that expresses the relative effect of an aircraft at a given configuration on a pavement structure for a specified standard subgrade strength. Conversely, the PCN is defined as a number that expresses the load-carrying capacity of a pavement for unrestricted operations. Therefore, the ACN-PCN system is structured so that a pavement with a particular PCN value can support unlimited repetitions of an aircraft that has an ACN equal to or less than the pavement's PCN value.

In the ACN/PCN method, the PCN, pavement type, subgrade strength category, tire pressure category, and evaluation method are all reported together. A code system has been implemented to allow an abbreviated presentation of the necessary information. The pavement type is abbreviated "R" for rigid (portland cement concrete [PCC]) and "F" for flexible (AC) pavements. Four subgrade categories, A, B, C, and D, indicate high, medium, low, and ultra-low subgrade strengths, respectively. The four tire-pressure categories, W, X, Y, and Z, indicate high, medium, low, and very low tire pressures, respectively. The evaluation methods are T for a technical evaluation and U for an evaluation based on the type and weight of the aircraft that commonly use the airfield. For example, the PCN code 90/F/C/W/T indicates that the PCN number is 90, that the pavement is flexible, that there is a low-strength subgrade, that high-pressure tires are allowed, and that a technical evaluation was performed to determine the PCN rating.

METHODOLOGY

As noted above, the pavement strength evaluation was accomplished in accordance with the Technical Method described in Advisory Circular 150/5335-5C. To complete the analysis, the following information was used for Runway 17-35:

Aircraft Traffic Volume: The traffic volume estimate was provided by Century West Engineering Corporation in terms of operations for Runway 17-35. The COMFAA 3.0 program includes a library of standard aircraft types, and we used the default gear weight for each aircraft in the aircraft fleet mix.

Pavement Structure: As noted earlier herein, the pavement thickness and subgrade support characteristics were estimated based on the FWD backcalculation results and core explorations.

The results of our PCN analysis are summarized in Form 5010 – Airport Master Record (Table 1B) and presented on Figure 1B of this appendix.

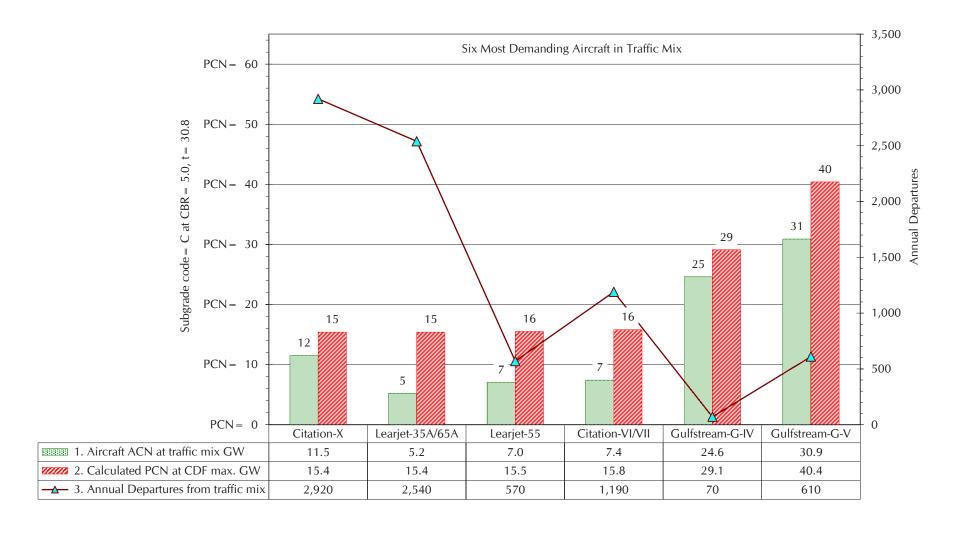
Reference

ICAO, 1999, Aerodrome standards – aerodrome design and operations, Annex 14, Third Edition.



Table 1B - FORM 5010 AIRPORT MASTER RECORD

		RE PRESSURE	МЕТНО	USED			ct info		
A Flexible Cate	gory (CBR 15)	W Unlimited	Usin	g Aircraft	Aurora State A	Airport			
B Flexible Cate	egory (CBR 10)	X 254 psi	● Tech	nnical					
C Flexible Cate	egory (CBR 6)	Y 145 psi							
D Flexible Cate	egory (CBR 3)	O Z 73 psi							
		AIRCRA	FT GEAR T	VDE IN TR	VEEIC MIX				
A Rigid Catego	ory (k 552 pci)	_			AI I IO WIIX				
B Rigid Catego	ory (k 295 pci)		D (dual wheel gear)						
C Rigid Catego	ory (k 147 pci)		D (dual wheel gear) 2D (dual tandem wheel gear) DDT or W/B (tandem gear under wing AND tandem gear under body)						
	ory (k 74 pci)	ZD (ddar tarider	ii wheel gear)		.g. B-747, A-340-60				
				A	irport LOC-ID	U,	AO		
Enter PCN	40				Pavement ID	RW	17-35		
Form 5040	Cross Weight	IF 3D or W	/B Gear C	hecked, #	#38 = PCN				
Form 5010 Data Element	Gross Weight and PCN	Please Ad							
#35 S gear	102	3D							
#36 D gear	143	2D/2D2							
#37 DT gear		2D/3D2W		Report Minimum					
#38 DDT gear			2D/3D2B Gross Weight						
#39 PCN	40/F/C/X/T								
		 #35 S	#36 D	#37 DT	#38 DDT				
Airport LOC-ID	Pavement ID	GW	GW	GW	GW	#39	PCN		
UAO	17-35	102	143			40/F/	/C/X/T		





PAVEMENT CLASSIFICATION CHART

SEP. 2019 JOB NO. 6289 FIG. 1B



Memo

To: Heather Peck, Projects and Programs Director, Oregon Department of Aviation

From: James Kirby, PE, Century West Engineering

Date: September 4th, 2020

Project: Aurora State Airport - Runway Pavement Considerations for Overweight Landings

Re: Evaluation and Recommendations

The Oregon Department of Aviation (ODA) has requested that Century West Engineering assess the existing information concerning Runway 17-35 at Aurora State Airport (UAO) and provide recommendations on further consideration of overweight landing requests there. A review of existing conditions, recent structural evaluation work, and qualitative factors related to the surface condition follows:

Existing conditions

The most recent ODA Pavement Evaluation Program (PEP) report prepared by Pavement Consultant Inc. (dated 2018) shows the existing Runway 17-35 pavement is comprised of two major sections. The largest being the 4,100' long Northern section of the runway, first constructed in 1943. The 900' long Southern extension was constructed in 1993. During the last major project in 2005, the entire length of the runway received a 2" to 3" overlay.

The PEP reports that the pavement surface of Runway 17-35 is in "satisfactory" condition with a weighted average Pavement Condition Index of 81. The primary distresses present on the runway are low- to medium-severity longitudinal cracking, low-severity weathering, and isolated low-severity alligator cracking. The longitudinal cracking is located primarily at paving joints created during the 2005 overlay project and sealed most recently in August of 2020. The alligator cracking is located in the gear path for the larger business jet aircraft using the airport.

When design for the 2005 project was being contemplated, FAA had limited the structural capacity input used in the design to 30,000 lbs (single wheel main gear) and 45,000 lbs (dual wheel main gear). It was determined that the existing pavement met those design criteria and as that project was not intended to increase runway capacity, the overlay was limited in depth. The 2" to 3" overlay was able to address surface conditions and combined with milling, extend the overall pavement section's life considerably. No additional structural testing of the final section was conducted at that time and as a result, the



current Airport Master Record (FAA Form 5010) lists the 30,000 lbs single wheel and 45,000 lbs dual wheel numbers as the gross weight limitations for the runway pavement.

Recent Structural Evaluations

In August of 2019, GRI performed a pavement evaluation of Runway 17-35 at UAO to determine the existing Pavement Classification Number (PCN). That project included review of ODA historical pavement records, falling weight deflectometer testing, pavement cores, and related analysis. The guidance provided in FAA Advisory Circular 150/5335-5C, Standardized Method of Reporting Airport Pavement Strength – PCN, was used to calculate the final PCN based on this work.

The reported PCN indicated that the existing pavement's structural capacity was greater than the 30,000 lbs single wheel and 45,000 lbs dual wheel numbers published in the Airport Master Record. GRI recommended that the single-wheel, main-gear and dual-wheel, main-gear aircraft gross weights be increased to 102,000 and 143,000 lbs, respectively based on the new PCN calculation. They hypothesized in their report that the 2005 overlay resulted in additional pavement section depth that likely increased the structural capacity. As design thicknesses for various portions of the pavement section are rounded up and factors of safety are built into the design process, these likely factored into the existing structure having increased capacity over the design numbers as well.

Overweight Landings

For aircraft exceeding the published pavement strength ratings, ODA requires submission of a Weight Limit Waiver Request and Liability Release Form prior to use of the airport. This anticipates that individual landings and takeoffs will be considered in light of the Runway strength rating and may be allowed on an individual basis. There have been a number of such requests approved in the last five years from operators of Gulfstream aircraft such as the GIV, GV, and GVI as well as Global Express aircraft.

The PCN calculation which yielded the GRI recommendation to increase the gross weight limits for the Runway does have some caveats that need to be considered. It should be noted that the PCN system is used as a method for airport operators to determine whether or not individual aircraft operations may be acceptable on their pavements. As such, it does not provide a mechanism to evaluate the cumulative damage from repeated aircraft operations of a specific type, size or configuration. In short, it does not provide a substitute for a pavement design or evaluation of changes in fleet mix, each which must be considered separately.

We looked at a representative fleet mix to see if an additional large aircraft might significantly reduce pavement life. Taking into account GRI's pavement strength assessment, it is unlikely that isolated operations of the aircraft that have made requests for overweight landings previously would significantly reduce the pavement life. Those aircraft gross takeoff weights are under the calculated pavement strengths so the effect of individual operations would be minimal.



However, large shifts in fleet mix to heavier aircraft should be considered carefully in light of the cumulative effect that major fleet changes have on pavement life. To evaluate the effect, a fleet mix could be created for the airport that included all operations broken out by specific aircraft type and configuration. Then that fleet mix could have one or more aircraft of interest added to the mix with their proposed operational counts and the cumulative effects on the pavement section could be quantified. The concerns noted by GRI in their report about the condition of the existing overlay however, preclude the use of that approach in any meaningful way.

Other Considerations

GRI noted low severity alligator cracking within the gear paths that warranted further examination. Pavement cores were drilled in those areas and the cracking was found to be top-down. GRI also noted delamination of the top course of asphalt (from the 2005 overlay). This type of cracking and delamination is indicative of shear stresses at the pavement surface from aircraft wheel loading during landing and hard braking.

These observations make looking at an individual aircraft's cumulative effect on pavement life problematic as those effects may not result in the most likely failure mode for the runway pavement. The FAA does not have an accepted approach for modelling shear stresses or delamination of overlays in a quantifiable way. Variability in the degree of delamination over the runway surface also presents a unique problem. We can examine what operations may make those situations worse however. Surface shear stresses result when aircraft tires contact the pavement surface and significant friction forces are generated. Examples are initial contact with the pavement surface at the touchdown point and hard wheel braking during rollout. Aircraft with large tire contact areas and heavier weights would be worse in this regard. Even lighter aircraft such a DC-3 when fitted with larger tires put the runway overlay at greater risk for shear failure due to their larger tire contact area.

Recommendations

Evaluation of waiver requests for aircraft exceeding the existing published pavement strength ratings provides ODA with a valuable tool to control further runway degradation. However, a qualitative approach is likely the best way to maintain overall pavement condition as long as possible when overweight operations are being considered. Individual or limited operations of aircraft with gross weights over the published maximums and under those weights indicated by the PCN calculations are likely negligible. Significant additional operations of aircraft in that weight range may warrant additional and specific study. We would also recommend that any overweight landing request be considered in light of the potential for shear stress failures in the form of overlay delamination and FOD generation from low-severity alligator cracking worsening on the runway.

In general, we would recommend that the runway be inspected more frequently to monitor pavement conditions at those locations where alligator cracking was noted. This would include the wheel paths along the length of the runway as well as the width of the runway in the landing areas at both ends. If



worsening alligator cracking, significant new transverse cracking, random cracking, or FOD generation is noted, further pavement inspection and assessment would be recommended as well.

Finally, we would also recommend ODA consider putting together a formal action plan for what steps would be taken should a surface failure occur. Should a failure happen, at best, significant FOD would be generated requiring shutdown and cleanup. At worst, a catastrophic failure along the weakened delamination plane may displace part of the runway surface and require a lengthier shutdown and significant repair. Coordinating with potential repair contractors or other local agency resources (ODOT, Marion county road crews, etc.) that might be brought in to address an immediate pavement need is an important consideration in reducing runway closure length.



MEMORANDUM

To: James Kirby, PE / Century West Engineering **Date:** June 8, 2021

GRI Project No.: 6488-A

From: Lindsi Hammond, PE

Re: Pavement Evaluation

Aurora Airport Runway 17/35 Remaining Structural Life Evaluation

Aurora, Oregon

As requested, GRI performed engineering analyses to determine the remaining structural life of Runway 17/35 at Aurora State Airport (UAO) in support of the Oregon Department of Aviation (ODA). This work was completed as a follow-up to the report titled "Pavement Classification Number (PCN) Evaluation of Runway 17-35," issued on November 12, 2019 (2019 PCN Report). As discussed in Federal Aviation Administration (FAA) Advisory Circular 150/5335-5C titled Standardized Method of Reporting Airport Pavement Strength – PCN, the PCN system has significant limitations such that the analysis consolidates the entire fleet mix into one representative aircraft and that the PCN should not be used to replace a structural evaluation or pavement design due to the complex nature and engineering judgment required beyond the outputs of the FAA software programs.

Our work included reviewing relevant ODA records for Runway 17/35, performing a multilayered backcalculation analysis using the falling weight deflectometer (FWD) data that were used to assist us in delivering our 2019 PCN Report, and evaluating the structural remaining life in general accordance with the FAA Advisory Circular 150/5320-6F, *Airport Pavement Design and Evaluation*, and the FAA pavement evaluation software, FAARFIELD (FAA Rigid and Flexible Iterative Elastic Layered Design) v1.42. Additional background data and analysis results are provided in Appendices A and B, respectively.

STRUCTURAL LIFE OF EXISTING PAVEMENT

The structural life of the existing pavement is calculated by the FAA design procedure based on traffic loading (i.e., aircraft fleet mix), structural properties of the existing pavement (thickness and modulus), and subgrade strength, as determined from investigation and testing of the pavement materials and subgrade soils. The structural life calculated in this manner only applies to the amount of time the existing pavement could support the forecasted traffic loading until its structural capacity decreases to the extent strengthening or reconstruction is required. Structural life does *not* account for deterioration in surface conditions or factors that can affect the integrity or functional life of the pavement system.



PAVEMENT FUNCTIONAL LIFE/PAVEMENT INTEGRITY

Pavement functional life is the period before the surface condition deteriorates to the state where there is significant potential for foreign object debris (FOD), which is the primary factor controlling the need for rehabilitation.

The functional life and integrity of asphalt concrete (AC) pavements are primarily controlled by 1) surface cracking that originates at the pavement surface and is typically confined to the upper pavement layers of the pavement system, 2) joint cracking, or 3) delamination of AC layers that can influence accelerated deterioration. Surface cracking may occur due to thermally induced movement, moisture exposure, and/or hardening of asphalt cement due to oxidation. Traffic loading, particularly with high tire pressures and heavily weighted aircraft, can initiate surface cracking and be an exacerbating factor in propagation and deterioration, especially when the upper AC layers exhibit delamination. In addition to the above factors, joint cracking is often caused by reduced compaction near the joint or mechanical and temperature segregation during asphalt construction.

ANALYSIS

We evaluated the remaining structural life of Runway 17/35 based on four traffic-loading scenarios, which included 1) current aircraft fleet mix; 2) current aircraft fleet mix plus 64 monthly operations of a Gulfstream G650ER (G650ER) at 103,600 pounds; 3) current aircraft fleet mix plus 64 monthly operations of a G650ER at 83,500 pounds; and 4) current aircraft fleet mix plus 64 monthly operations of a G650ER at 75,000 pounds. The aircraft fleet mix is provided in Tables 1A and 2A of Appendix A.

RESULTS

Based on the current aircraft fleet mix, the existing runway should be scheduled for rehabilitation within the next 10 years (e.g., sooner than the estimated remaining structural life). Table 1 shows our recommended timeframe for rehabilitation or reconstruction based on the results of the analysis in combination with the current integrity/functional life of the pavement system. Runway 17/35 exhibits delamination of the upper 2 inches to 3 inches of AC. In our opinion, the delamination in combination with the presence of fatigue cracking contributes to recommending a reduced remaining structural life. Additional details are provided in Appendix B.

Table 1: RECOMMENDED TIME UNTIL REHABILITATION/RECONSTRUCTION

Current Fleet Mix	Additional G650ER Operations @ 103,600 pounds	Additional G650ER Operations @ 83,500 pounds	Additional G650ER Operations @ 75,000 pounds
10 years	0 years	Within 5 years	Within 10 years



LIMITATIONS

This memorandum has been prepared for use by the Oregon Department of Aviation and Century West Engineering Corporation and should not be relied upon by any other entity without the written permission of an authorized representative. The scope is limited to the specific project and location described herein, and our description of the project represents our understanding of the significant aspects of the project relevant to the analysis of the pavements at the time of publication. In the event any changes in the parameters as outlined in this memorandum are planned, we should be given the opportunity to review the changes and modify or reaffirm the conclusions and recommendations of this memorandum in writing.

The conclusions and recommendations submitted in this memorandum are based on the data obtained from the subsurface explorations referenced in this memorandum and other sources of information discussed herein. In the performance of subsurface investigations, specific information is obtained at specific locations at specific times. However, it is acknowledged variations in soil conditions may exist between exploration locations. This memorandum does not reflect any variations that may occur between these explorations. The nature and extent of variation may not become evident until construction and/or after additional field explorations. Additionally, our work has been performed in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing under similar conditions in the locale. No other warranty, expressed or implied, is made.

Please contact the undersigned if you have any questions.

Submitted for GRI,

88693PE

dsilan

OREGON

Lindsi Hammond, PE

ALEYCE HA Expires 06-2021

Principal

Todd Scholz, PE

Principal

This document has been submitted electronically.

6488-A UAO REMAINING STRUCTURAL LIFE MEMO



APPENDIX A

Pavement Evaluation Background Data



APPENDIX A

PAVEMENT EVALUATION BACKGROUND DATA

A.1 BACKGROUND

Based on the information provided in the ODA pavement evaluation/maintenance management program report prepared by Pavement Consultants Inc. in 2018, the runway was constructed in two phases. The 4,100 foot-long segment on the north end of the runway was first constructed in 1943, which is referred to herein as Analysis Unit 1. In 1993, a 900-foot-long extension was built to the south, which is referred to herein as Analysis Unit 2. The locations of Analysis Units 1 and 2 are shown on Figure 1A. The last major rehabilitation on the runway was conducted in 2005 and generally consisted of a 2- to 3-inch-thick overlay. Based on the construction history provided in the 2018 ODA report, the runway was constructed with 6 inches to 8 inches of asphalt concrete (AC), whereas the results from the 2019 core explorations found 8¾ inches to 9 inches of AC. The aggregate base and subbase ranges from 15 inches to 49 inches, which was not field-verified during the 2019 project.

As discussed in the 2019 PCN Report, GRI observed isolated areas of low-severity fatigue cracking (i.e., alligator cracking) within the aircraft landing gear paths on Runway 17/35. Also, the extracted core specimens exhibited delamination (separation of asphalt concrete [AC] layers) at a depth that generally agrees with the thickness of the 2005 overlay. The cores also showed top-down cracking to the same depth as the delamination. The presence of these distresses indicates material degradation, which can impact the integrity of the pavement system and structural performance.

A.2 TRAFFIC LOADING

The 2019 PCN Report listed aircraft traffic-volume data consisting of the number of operations (i.e., either an arrival or departure) for Runway 17/35 in 2018 from the FAA Traffic Flow Management System Counts (TFMSC). Our traffic-loading estimate is based on escalating the traffic volumes to the year 2041 for a 20-year period using an annual growth rate of 1.58% per year, which is based on the aviation forecasts provided in the current master plan for UAO (WHPacific, 2012). The aircraft fleet mix is provided in Table 1A. Based on the existing aircraft fleet mix, over 99%, based on the number of annual departures, operate at a gross takeoff weight of less than 50,000 pounds.



Table 1A: CURRENT AIRCRAFT TYPES AND DEPARTURE VOLUMES

	Gross				Values Entered int	o FAARFIELD
Aircraft Type	Takeoff Weight, pounds	Design Aircraft for FAARFIELD	2021 Annual Operations	2041 Annual Operations	Equivalent Airplane	2021 Annual # of Departures
Gulfstream G600	91,600	Gulfstream G-V	3	3	Gulfstream G-V	2
Gulfstream V	76,850	Gulfstream G-IV	3	3	Gulfstream G-IV	4
Gulfstream IV	73,200	Gulfstream G-IV	3	3	Guilstream G-1V	4
Dassault Falcon 900	45,500	Falcon-900	72	84	Falcon-900	42
Bombardier Challenger 600	45,100	Challenger CL- 604	61	71	Challenger CL-604	91
Bombardier Challenger 300	38,850	Challenger CL- 604	93	110	Challenger CL-004	91
Dassault Falcon 2000	41,000	Falcon-2000	36	42	Falcon-2000	21
Dassault Falcon 50	37,480	Falcon-50	290	338	Falcon-50	216
Dassault Falcon 20	28,650	Falcon-50	80	98	Falcon-50	210
Cessna Citation 750	36,600	Citation X	110	128	Citation X	150
Cessna Citation 680	30,775	Citation X	145	169	Citation X	150
Hawker 800	28,000	Hawker-800	36	42	Hawker-800	21
Gulfstream G150	26,100	D-35	84	98	D-35	49
Astra 1125	24,650	D-30	101	118	D-30	59
Cessna Citation 650	22,000	Citation VI/VII	103	120	Citation VI/VII	61
Learjet 60	23,500	Learjet-55	32	37		
Learjet 55	21,500	Learjet-55	5	6	Learjet-55	30
Learjet 75	21,500	Learjet-55	13	15		
Learjet 45	20,500	Learjet-35A/65A	116	135		
Learjet 35	18,000	Learjet-35A/65A	9	10	Learjet-35A/65A	131
Learjet 31	15,500	Learjet-35A/65A	97	113		
Cessna Citation 560	20,000	Citation 550B	738	860	Citation 550B	561
Cessna Citation 550	13,300	Citation 550B	223	260	Citation 330b	301
Phenom 300/ Embraer 300	17,968	D-25	59	69	D-25	35

In addition to the current aircraft fleet mix we also evaluated the impact of adding a G650ER at three different weights as shown in Table 2A.



Table 2A: ADDITIONAL AIRCRAFT TYPES AND DEPARTURE VOLUMES

	Gross				Values Entered into FAARFIELD		
Aircraft Type	takeoff Weight, pounds	Design Aircraft for FAARFIELD	2021 Annual Operations	2041 Annual Operations	Equivalent Airplane	2021 Annual # of Departures	
Gulfstream G650ER	103,600	Gulfstream G-V	768	895	Gulfstream G-V	448	
Gulfstream G650ER	83,500	Gulfstream G-V	768	895	Gulfstream G-V	448	
Gulfstream G650ER	75,000	Gulfstream G-V	768	895	Gulfstream G-V	448	

A.3 BACKCALCULATION

A.3.1 FWD Data

Falling weight deflectometer (FWD) tests were conducted by GRI on August 20, 2019, using our KUAB Model 150 FWD. The annual reference calibration for the FWD was accomplished in October 2019 at the KUAB manufacturing facility in Savoy, Illinois.

The FWD testing on Runway 17/35 was accomplished along test lines located at 7 feet west and 12 feet east of the runway centerline. The tests were completed at approximately 200-foot intervals within the keel section of the runway. This work was performed as a part of the "Pavement Classification Number (PCN) Evaluation of Runway 17-35" project, which the report was issued on November 12, 2019 (2019 PCN Report).

A.3.2 Overview of Backcalculation Analysis Procedure

The FWD deflection data were analyzed to backcalculate the in-situ equivalent elastic moduli of the pavement layers and subgrade soil following the guidelines of ASTM D5858 and Federal Aviation Administration (FAA) Advisory Circular 150/5370-11B. This analysis was accomplished using our PAVBACK iterative, elastic, layered backcalculation analysis software. The software calculates deflections using the Boussinesq-Odemark method of an equivalent thickness (Ullidtz, 1998). Pavement layer moduli are determined through an iterative search process using the MINPACK-1 (More et al., 1980) version of the Levenberg-Marquardt non-linear least-squares minimization algorithm with the objective of minimizing the root mean squared deflection error (RMSE), as computed by:

$$RMSE = \sqrt{\frac{1}{n} \sum_{j=1}^{n} (d_j - w_j)^2}$$
 (1)

where:

dj = Measured deflection at sensor j; (j = 1, ..., n = number of sensors)

wj = Calculated deflection at sensor j



PAVBACK solutions were validated by comparing the calculated and measured values of asphalt tensile strain and subgrade compressive strain/stress. The deflection test data and corresponding measured values of strain and stress used for the validation were obtained from data published in a report about backcalculation analysis of deflection tests conducted on an instrumented pavement test section (Ullidtz, ASTM STP 1375, 2000). The reported deflection measurements were inputted into PAVBACK to backcalculate the moduli of the pavement layers and subgrade in the test section. The moduli backcalculated by PAVBACK were then used to calculate asphalt tensile strain and subgrade compressive strain/stress for the FWD load corresponding to the reported measured stress and strain values. The calculated strains and stress were found to agree nearly exactly with the reported measured strain and stress values (within ±10% of the measured values).

A.3.3 Backcalculation Models

We modeled the pavement as a multilayered elastic three-layered system to backcalculate the equivalent elastic moduli (as applicable) of the AC, aggregate base (AB) and/or aggregate subbase (ASB), and subgrade soil. We used the pavement layer thicknesses reported in our 2019 PCN Report from the shallow core explorations in the backcalculation analysis. Furthermore, the data was separated into two analysis units based on the differing construction as discussed previously.

The multilayered backcalculation analysis uses mathematical optimization techniques to calculate the equivalent elastic modulus values of the pavement layers and subgrade soil to minimize the difference between deflections calculated according to the analysis model and the deflections measured in the field. This analysis is conducted by an iterative approach beginning with an assumed set of layer moduli. Pavement surface deflections are calculated according to elastic layer theory using these initial layer moduli. The computed deflections are compared with the measured deflections, and the initial layer moduli are adjusted to reduce the differences between the calculated and measured deflections. The adjusted moduli are then used to start the next analysis iteration. The iteration process continues until the computed, and measured deflections match within a specified tolerance or until the adjustment to the solution values is less than a specified tolerance. The "goodness of fit" between the measured and computed deflections is measured by the RMSE, which is calculated using the percent difference between the measured and calculated deflections relative to the measured deflection and is roughly a measure of the relative percent error per deflection sensor.

For the analysis, we used the average subgrade modulus less one standard deviation from the backcalculation results to estimate the design subgrade moduli for each analysis unit.

A.3.4 Backcalculation Analysis Results

The backcalculation analysis results are tabulated in Table 3A for Runway 17/35. These results include the layer thicknesses, backcalculated moduli with the AC moduli normalized to a



pavement temperature of 82 °F and loading frequency of 2 Hertz (discussed below), equivalent P-401 AC thicknesses (discussed below), and the RMSE values of the backcalculation solutions.

The backcalculated AC moduli were normalized using the Asphalt Institute's predictive equation (Finn et al., 1982) to correspond to a pavement temperature of 82 °F and loading frequency of 2 Hertz. These normalization conditions are based on the design pavement temperature for UAO and the equivalent loading frequency of taxiing aircraft as determined by the U.S. Army Corps of Engineers (COE) airfield design procedure, which is the basis for the Advisory Circular 150/5320-6F design procedures. The modulus of new AC for the same normalizing conditions is 200 kips per square inch (ksi), as predicted by the COE airfield design procedure. This is the same value as the modulus assigned to P-401 AC surface course in the FAARFIELD software. Therefore, backcalculated normalized AC moduli of less than 200 ksi indicate the structural value of the existing AC is lower than the new P-401 AC surface course.

Since the FAARFIELD software does not allow for changing the modulus of AC surface course or base course except by entering the AC as an undefined material, the backcalculated normalized moduli for existing AC cannot be directly used in structural analysis by the FAARFIELD software. In order to overcome this limitation, the thickness of existing AC with a normalized backcalculated modulus of less than 200 ksi was adjusted (reduced from the actual thickness) so the flexural stiffness of the adjusted AC section at a modulus of 200 ksi is the same as the flexural stiffness of the actual AC section at the normalized backcalculated moduli. The adjusted thickness is calculated by the following equation derived from the method of equivalent thickness:

$$T_{eq} = T_{ac} \left(\frac{E_{ac}}{200} \right)^{1/3} \tag{2}$$

where:

 T_{eq} = Equivalent P-401 AC (at 200-ksi modulus) thickness, inches

 T_{ac} = Actual thickness of AC, inches

 E_{ac} = Backcalculated AC modulus normalized to 82 °F and 2 Hertz, ksi \leq 200 ksi

 ${\left({^{E_{ac}}\!/_{200}} \right)^{^{1}\!/_{\!3}}}$ = AC thickness to P-401 thickness conversion factor

This adjustment ensures the computed stresses and strains for layers below the AC layer reflect the reduced structural capacity of the existing AC, corresponding to its normalized backcalculated modulus being lower than the 200-ksi modulus assigned by FAARFIELD for AC surface course. Note that the thickness adjustment is only applied downward and not upward; therefore, the structural analysis becomes more conservative when the normalized backcalculated modulus of



AC is greater than 200 ksi. The calculated AC thickness conversion factors and equivalent P-401 AC thicknesses are included with the tabulated backcalculation analysis results.

References

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Table 3A - MULTILAYER BACKCALCULATION ANALYSIS SUMMARY RUNWAY 17/35: AURORA STATE AIRPORT (UAO)

Runway 17/35: Aurora State Airport (UAO)

Based on FWD Testing Conducted: 8/20/2019 [Report Titled "Pavement Classification Number (PCN) Evaluation of Runway 17-35" issued on November 12, 2019] Start Station: North edge of runway, 10+00

FWD Test	Test Station	Test Line	Core Exploration	Analysis Unit	Center Deflection (D ₀), mils	AC Thickness, inches	AB/ASB Thickness, inches	AC Modulus @ 82°F & 2 Hz, psi	AB Modulus, psi	Subgrade M _R at 6 psi Deviator Stress, psi	Existing AC Thickness to P-401 Thickness Conversion Factor	Equivalent P-401 AC (@ 200 ksi) Thickness, inches
1	10+50	7 feet w		1	28.54	9.00	15.00	199,573	34,592	10,402	1.00	8.99
2	12+50	7 feet w		1	25.28	9.00	15.00	129,400	64,221	15,441	0.86	7.78
3	14+49	7 feet w		1	30.42	9.00	15.00	159,107	36,513	11,553	0.93	8.34
4	16+51	7 feet w		1	29.35	9.00	15.00	166,815	39,633	11,570	0.94	8.47
5	18+50	7 feet w		1	24.65	9.00	15.00	182,973	44,396	12,902	0.97	8.74
6	20+56	7 feet w		1	27.93	9.00	15.00	136,993	39,213	11,768	0.88	7.93
7	22+50	7 feet w		1	25.72	9.00	15.00	158,978	49,948	14,630	0.93	8.34
8	24+51	7 feet w		1	26.54	9.00	15.00	155,524	37,967	12,567	0.92	8.28
9	26+53	7 feet w		1	26.28	9.00	15.00	122,045	50,461	15,004	0.85	7.63
10	28+55	7 feet w		1	26.82	9.00	15.00	152,341	47,589	14,486	0.91	8.22
11	30+54	7 feet w		1	26.27	9.00	15.00	144,662	60,171	13,228	0.90	8.08
12	32+54	7 feet w		1	30.95	9.00	15.00	140,076	44,596	10,155	0.89	7.99
13	34+52	7 feet w		1	36.96	9.00	15.00	61,910	45,388	9,847	0.68	6.09
14	36+57	7 feet w		1	32.41	9.00	15.00	121,697	41,002	10,365	0.85	7.63
15	38+52	7 feet w		1	28.76	9.00	15.00	135,420	42,673	10,556	0.88	7.90
16	39+51	7 feet w	B-2	1	34.09	9.00	15.00	82,735	56,700	9,726	0.75	6.71
17	40+51	7 feet w		1	27.27	9.00	15.00	141,083	48,581	10,489	0.89	8.01
18	42+51	7 feet w		1	31.58	9.00	15.00	121,645	39,640	11,108	0.85	7.63
19	44+51	7 feet w		1	29.21	9.00	15.00	105,805	45,644	11,314	0.81	7.28
20	46+50	7 feet w		1	29.41	9.00	15.00	124,285	36,411	11,087	0.85	7.68
21	48+52	7 feet w		1	28.25	9.00	15.00	138,708	37,945	14,129	0.89	7.97
22	50+52	7 feet w		2	39.77	8.75	15.00	60,512	33,025	8,814	0.67	5.87
23	52+50	7 feet w		2	34.37	8.75	15.00	113,342	28,356	9,367	0.83	7.24
24	54+51	7 feet w		2	44.23	8.75	15.00	80,066	18,997	6,713	0.74	6.45
25	56+40	7 feet w	5.4	2	37.32	8.75	15.00	87,111	21,059	9,796	0.76	6.63
26	56+81	7 feet w	B-1	2	35.88	8.75	15.00	117,034	20,889	7,615	0.84	7.32
27	58+50	7 feet w		2	35.45	8.75	15.00	91,355	22,326	9,512	0.77	6.74
28	11+50	12 feet e		1	25.22	9.00	15.00	172,552	54,943	12,541	0.95	8.57
29	13+50	12 feet e		1	30.01	9.00	15.00	147,564	43,263	11,399	0.90	8.13
30	15+51	12 feet e		1	30.03	9.00	15.00	148,549	39,794	9,781	0.91	8.15
31	17+53	12 feet e	D 2	1	28.42	9.00	15.00	125,716	42,941	11,645	0.86	7.71
32	19+41 21+50	12 feet e 12 feet e	B-3	1	34.02 21.06	9.00 9.00	15.00 15.00	80,430 185,230	34,690 57,106	10,977 17,720	0.74 0.97	6.64 8.77
33	23+52	12 feet e		1					·		0.90	
35	25+52	12 feet e		1	25.55 21.98	9.00 9.00	15.00 15.00	145,745 149,035	53,157 74,897	13,364 14,811	0.90	8.10 8.16
36	27+51	12 feet e		1	26.27	9.00	15.00	112,695	49,426	14,236	0.83	7.43
37	29+50	12 feet e		1	34.66	9.00	15.00	102,930	33,377	11,837	0.80	7.43
38	31+52	12 feet e		1	27.24	9.00	15.00	124,820	61,287	10,942	0.85	7.69
39	33+49	12 feet e		1	26.34	9.00	15.00	145,949	51,343	11,421	0.90	8.10
40	35+53	12 feet e		1	24.64	9.00	15.00	149,184	53,878	14,477	0.91	8.16
41	37+51	12 feet e		1	29.65	9.00	15.00	136,435	47,933	10,835	0.88	7.92
42	39+50	12 feet e		1	25.27	9.00	15.00	171,828	51,898	11,501	0.95	8.56
43	41+51	12 feet e		1	25.80	9.00	15.00	166,029	48,784	13,236	0.94	8.46
44	43+50	12 feet e		1	27.58	9.00	15.00	156,236	46,762	11,913	0.92	8.29
45	45+51	12 feet e		1	26.22	9.00	15.00	141,024	43,126	12,250	0.89	8.01



Table 3A - MULTILAYER BACKCALCULATION ANALYSIS SUMMARY RUNWAY 17/35: AURORA STATE AIRPORT (UAO)

FWD Test		Test Line	Core Exploration	Analysis Unit	Center Deflection (D₀), mils	AC Thickness, inches	AB/ASB Thickness, inches	AC Modulus @ 82°F & 2 Hz, psi		psi Deviator Stress,	_	Equivalent P-401 AC (@ 200 ksi) Thickness, inches
46	47+54	12 feet e		1	28.02	9.00	15.00	124,960	37,437	11,825	0.85	7.69
47	49+51	12 feet e		1	27.34	9.00	15.00	139,375	36,694	12,606	0.89	7.98
48	51+53	12 feet e		2	30.35	8.75	15.00	137,690	27,259	11,238	0.88	7.73
49	53+55	12 feet e		2	31.95	8.75	15.00	131,769	24,002	10,326	0.87	7.61
50	55+50	12 feet e		2	36.26	8.75	15.00	88,169	25,673	9,761	0.76	6.66
51	57+51	12 feet e		2	32.67	8.75	15.00	125,325	19,556	9,341	0.86	7.49

Abbreviations:

M_R = Resilient Modulus; psi = Pounds per Square Inch; ksi = Kips per Square Inch; AC = Asphalt Concrete; AB = Aggregate Base; ASB = Aggregate Subbase; e = east of centerline; w = west of centerline; Hz = Hertz; °F = Degree Fahrenheit; PMP = Pavement Management Program

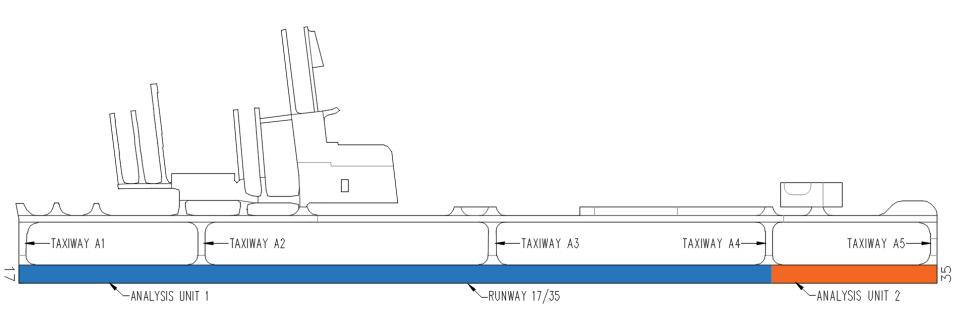
Statistical Summary

	,											
									Average	Average Equivalent		
							Average AC		Subgrade M _R at	P-401 AC		
Structural			PAVER PMP	Average D0,	Average AC	Average AB/ASB	Modulus @ 82°F	Average AB	6 psi Deviator	(@ 200 ksi)		
Unit#	From Sta	To Sta	Unit	mils	Thickness, inches	Thickness, inches	& 2 Hz, psi	Modulus, psi	Stress, psi	Thickness, inches		
1	0+00	49+51	R17AU-01	28.10	9.00	15.00	139,221	46,488	12,235	7.9		
2	0+00	58+50	R17AU-02	35.83	8.75	15.00	103,237	24,114	9,248	7.0		
All	0+00	58+50	ALL	29.61	8.95	15.00	132,165	42,101	11,650	7.7		

Design Subgrade Resilient Modulus

Structural Unit #	From	То	PAVER PMP Unit	Average Subgrade M _R at 6 psi Deviator Stress, psi	Standard Deviation, psi	Average Subgrade Less One Standard Deviation, psi	
1	10+50	49+51	R17AU-01	12,235	1,800	10,435	7.0
2	50+52	58+50	R17AU-01	9,248	1,294	7,955	5.3
All	0+00	58+50	ALL	11,650	2,081	9,569	6.4









SITE PLAN

JUN. 2021

JOB NO. 6488

FIG. 1A



APPENDIX B

Remaining Structural Life Analysis



APPENDIX B

REMAINING STRUCTURAL LIFE ANALYSIS

B.1 REMAINING STRUCTURAL LIFE

We estimated the remaining pavement life of Runway 17/35, also referred to as "remaining structural life," using the FAA evaluation procedure and Version 1.42 of the FAARFIELD pavement-design software program. The results are based on the current traffic loading, growth rates, structural properties of the existing pavement (thickness and modulus), and subgrade stiffness determined from the previous pavement-core explorations and FWD deflection test data from the 2019 PCN Report, pavement and subgrade soils laboratory testing, and backcalculation analysis.

Remaining structural life of AC pavements is based on an analysis of the cumulative damage factor (CDF) for two modes of pavement failure: rutting due to excessive vertical compressive strain at the top of the subgrade, and fatigue cracking due to excessive horizontal strain in the bottom of the AC layer. Structural life calculated in this manner only applies to how long the existing pavement would support the forecast aircraft fleet mix until its structural capacity decreases to the extent that strengthening, or reconstruction is required to avoid significant risk of structural damage by heavily loaded aircraft. Since structural life does not account for deterioration in the bound-pavement layer, pavement structures can have calculated structural lives well in excess of a typical design period. Furthermore, the results, even though they meet the desired remaining life, may not be realistic from a material-degradation standpoint due to the presence of delamination, stripping, and/or cracking distress.

We have presented the FAARFIELD outputs showing the calculated remaining structural life of Runway 17/35 on Figures 1B to 2B for Analysis Unit 1 and on Figures 3B to 4B for Analysis Unit 2 in this appendix. The results are also summarized in Table 1B below. We found that Analysis Unit 2, which encompasses the runway extension between Taxiways A4 to A5 has a significantly lower remaining structural life as compared to Analysis Unit 1. The shorter life is likely due to the thinner AC section and lower subgrade moduli. If a G650ER is added to the fleet mix, we calculated the remaining structural life to range from 1 year to 14 years, depending on the operational weight. We assume that the G650ER will require the full length of the runway to operate, and therefore Analysis Unit 2 controls the remaining structural life.

Additionally, due to the presence of delamination in the upper 2 inches to 3 inches of AC and cracking distress located in the landing gear path, it is our opinion, that the calculated remaining structural life results provided in Table 1B may be reduced due to the deteriorated condition of the AC. The addition of larger aircraft generally heavier than 50,000 pounds may further accelerate the pavement deterioration resulting in the development of foreign object debris (FOD) and



ultimately requiring rehabilitation sooner. In our opinion, under the current traffic loading without the operation of a G650ER the runway will require rehabilitation in approximately 10 years even though the remaining structural life is approximately 20 years. If the G650ER plans to operate on a regular basis at maximum gross weight (i.e., 103,600 pounds), we recommend rehabilitating the runway prior to operation because the runway will likely require structural strengthening. If the G650ER operates at a lower weight, we recommend planning a rehabilitation project within the next five years due to the condition of the AC. Table 1 presented above shows our recommended timeframe until rehabilitation/reconstruction, which is based on the results structural analysis results from FAARFIELD analysis and the functional condition of the runway materials.

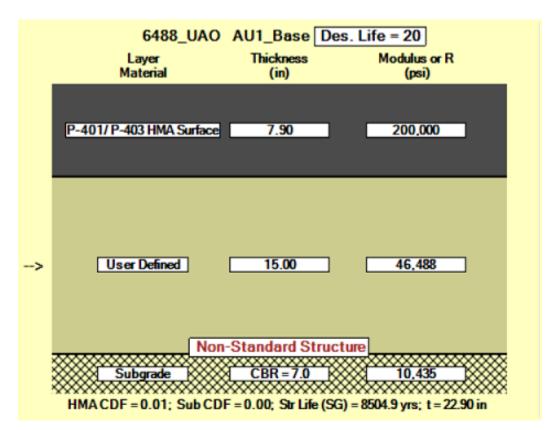
Table 1B: SUMMARY OF REMAINING STRUCTURAL LIFE RESULTS

			Remaining Life, years ^(a)									
	Analysis Unit	Current Fleet Mix	Additional G650ER Operations @ 103,600 pounds	Additional G650ER Operations @ 83,500 pounds	Additional G650ER Operations @ 75,000 pounds							
(1)	Runway 17/35 Taxiway A1 to A4	>20	>20	>20	>20							
(2)	Runway 17/35 Taxiway A4 to A5	>20	1	6	14							

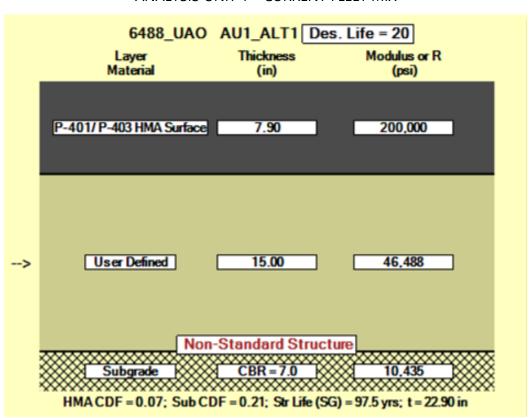
Note:

a) The remaining structural life may be well in excess of the reasonable timeframe that the runway may warrant rehabilitation or reconstruction from a material-degradation standpoint (i.e., delamination, stripping, or cracking distress).

We developed our results using limited subsurface condition data collected to assist us in developing the abovementioned 2019 PCN Report. The 2019 fieldwork only included three shallow core explorations, which were terminated at 24 inches below the ground surface. At each core exploration, we did not encounter subgrade. In order to refine the remaining life evaluation or to develop rehabilitation or reconstruction design recommendations, we recommend performing deeper borings in order to quantify the total aggregate base thickness and to classify the subgrade. The results from additional boring explorations may change the results of the remaining life results presented above.



ANALYSIS UNIT 1 – CURRENT FLEET MIX

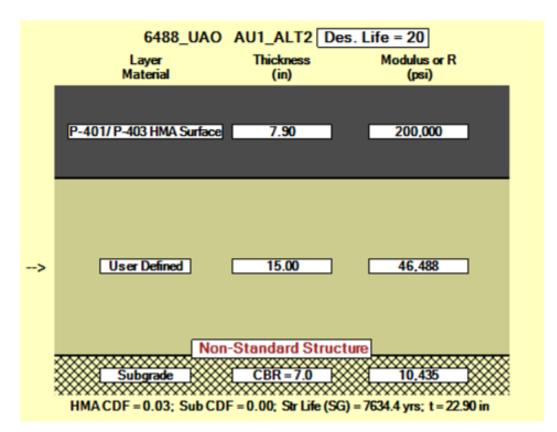


ANALYSIS UNIT 1 – CURRENT FLEET MIX + 64 MONTHLY OPERATIONS OF A G650ER @ 103,600 POUNDS

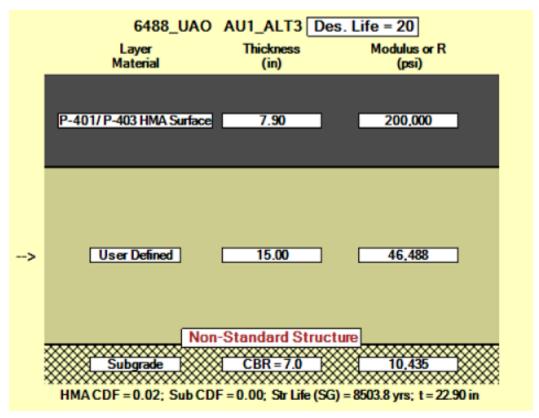


STRUCTURAL LIFE RESULTS

JUN. 2021 JOB NO. 6488 FIG. 1B



ANALYSIS UNIT 1 – CURRENT FLEET MIX + 64 MONTHLY OPERATIONS OF A G650ER @ 83,500 POUNDS

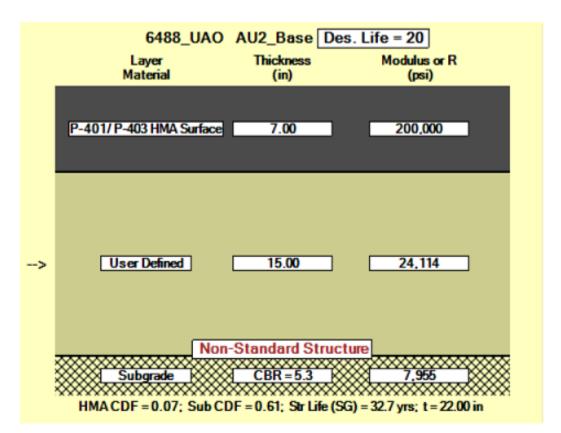


ANALYSIS UNIT 1 – CURRENT FLEET MIX + 64 MONTHLY OPERATIONS OF A G650ER @ 75,000 POUNDS

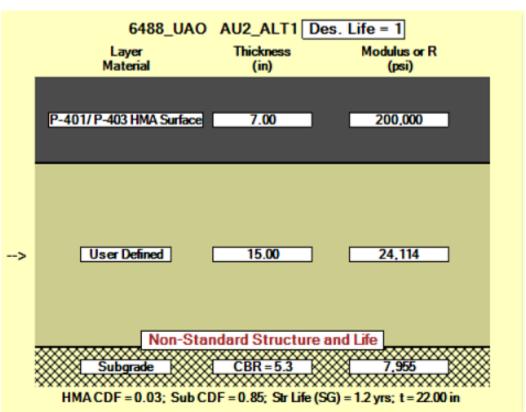


STRUCTURAL LIFE RESULTS

JUN. 2021 JOB NO. 6488 FIG. 2B



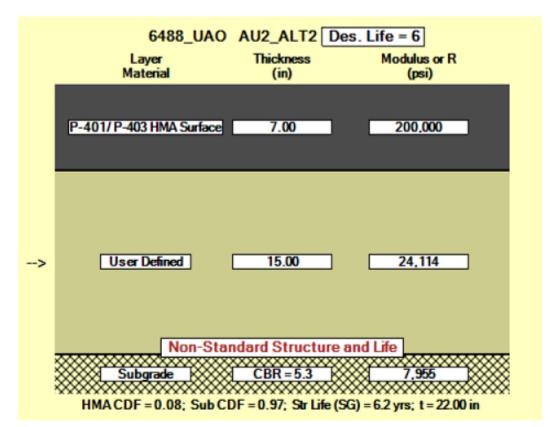
ANALYSIS UNIT 2 - CURRENT FLEET MIX



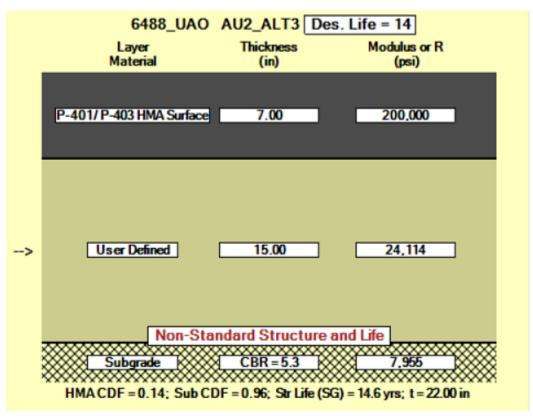
ANALYSIS UNIT 2 - CURRENT FLEET MIX + 64 MONTHLY OPERATIONS OF A G650ER @ 103,600 POUNDS



STRUCTURAL LIFE RESULTS



ANALYSIS UNIT 2 – CURRENT FLEET MIX + 64 MONTHLY OPERATIONS OF A G650ER @ 83,500 POUNDS



ANALYSIS UNIT 2 – CURRENT FLEET MIX + 64 MONTHLY OPERATIONS OF A G650ER @ 75,000 POUNDS



STRUCTURAL LIFE RESULTS

JUN. 2021 JOB NO. 6488 FIG. 4B